

BUILDING CODE
2018 INTERNATIONAL BUILDING CODE W/ STATE AMENDMENTS

<u>DEAD LOAD</u>	<u>LIVE LOAD</u>
SEE PLAN	SEE PLAN

GROUND SNOW (Pg) = 5.0 PSF
FLAT ROOF SNOW LOAD (Pf) = 3.5 PSF
SNOW EXPOSURE FACTOR (Ce) = 1.0
SNOW IMPORTANCE FACTOR (Is) = 1.0
SNOW THERMAL FACTOR (Ct) = 1.0

ULTIMATE WIND SPEED (3-SECOND GUST) = 110.0 MPH
 ALLOWABLE WIND SPEED (3-SECOND GUST) = 85.2 MPH
 WIND EXPOSURE CATEGORY = C
 INTERNAL PRESSURE COEFFICIENT = +/- 0.18
 COMPONENTS AND CLADDING PRESSURE:

ZONE		ULTIMATE					NOMINAL				
		EFFECTIVE AREA (SF)					EFFECTIVE AREA (SF)				
		4.0	20.5	50	100	500	2.0	20.3	50	100	500
ROOF	1	13.4	40.5	36.7	33.9	27.2	26.0	24.3	22.0	20.3	16.3
	2	57.2	53.5	48.7	45.0	36.4	34.3	32.1	29.2	27.0	21.9
	3	78.0	70.6	60.9	53.5	36.4	46.8	42.4	36.5	32.1	27.7
WALL	4	27.0	25.9	24.4	23.3	20.8	16.2	15.5	14.7	14.0	12.5
	5	33.2	31.0	28.1	25.9	20.8	19.9	15.5	16.9	15.5	12.5

NOTE: COMPONENTS AND CLADDING ELEMENTS NOT SPECIFICALLY DESIGNED ON THESE DRAWINGS SHALL BE DESIGNED TO THE WIND PRESSURES STIPULATED BY ASCE7-16 FOR THE TRIBUTARY AREA OF THE SPECIFIC COMPONENT.

SEISMIC IMPORTANCE FACTOR (I_e) = 1.10
OCCUPANCY CATEGORY = II
 $S_s = 0.193$ $S_1 = 0.078$
SITE CLASS = D (PER GEOTECH REPORT)
 $S_{ds} = 0.206$ $S_{d1} = 0.125$
SEISMIC DESIGN CATEGORY = B
SEISMIC ANALYSIS PROCEDURE = EQUIVALENT LATERAL FORCE

X-DIRECTION (PLAN E-W):
STRUCTURAL STEEL SYSTEMS NOT SPECIFICALLY DETAILED FOR SEISMIC
RESISTANCE
R = 3.0 Cs = 0.069

Y-DIRECTION (PLAN N-S):
STRUCTURAL STEEL SYSTEMS NOT SPECIFICALLY DETAILED FOR SEISMIC
RESISTANCE
R = 3.0 Cs = 0.069

X-DIRECTION BASE SHEAR = 21.4 KIPS (WIND CONTROLS)
Y-DIRECTION BASE SHEAR = 41.6 KIPS (WIND CONTROLS)

1. THE FOLLOWING NOTES APPLY TO ALL PROJECT RELATED STRUCTURAL DOCUMENTS, INCLUDING THIS SET OF DRAWINGS, FIELD SKETCHES, AND RESPONSES TO REQUESTS FOR INFORMATION (RFI'S), UNLESS OTHERWISE INDICATED. THEY ARE INTENDED TO SUPPLEMENT THE PROJECT SPECIFICATION'S MANUAL. IN CASE OF CONFLICT, THE MOST STRINGENT REQUIREMENT/CONDITION SHALL APPLY.
2. SPECIFIC NOTES AND DETAILS SHALL TAKE PRECEDENCE OVER GENERAL NOTES.
3. THE CONTRACTOR SHALL VERIFY ALL CONTRACT DOCUMENTS, SITE ELEVATIONS, DIMENSIONS, AND CONDITIONS PRIOR TO START OF WORK, AND SHALL NOTIFY THE ARCHITECT/ENGINEER OF ANY DISCREPANCIES OR INCONSISTENCIES.
4. THE CONTRACT STRUCTURAL DOCUMENTS, INCLUDING DRAWINGS AND SPECIFICATIONS, REPRESENT THE FINAL CONDITION OF THE COMPLETED STRUCTURE. THE STRUCTURE IS CONSIDERED UNSTABLE UNTIL ALL FRAMING MEMBERS AND THEIR CONNECTIONS ARE INSTALLED. THE CONTRACTOR IS RESPONSIBLE FOR PROVIDING THE DESIGN, SAFETY, ADEQUACY, AND STABILITY OF ANY TEMPORARY SHORING AND BRACING THAT MAY BE REQUIRED FOR THE CONSTRUCTION OF THE STRUCTURE. MEANS AND/OR METHODS OF CONSTRUCTION ARE NOT THE RESPONSIBILITY OF THE ENGINEER, AND ARE TO BE PROVIDED BY THE CONTRACTOR FOR ALL PHASES OF CONSTRUCTION.
5. THE CONTRACTOR SHALL NOTIFY THE ARCHITECT/ENGINEER IN WRITING OF ANY DEVIATION FROM THE CONTRACT DOCUMENTS. THE CONTRACTOR SHALL NOT BE RELIEVED OF THE RESPONSIBILITY OF SUCH DEVIATION BY THE ENGINEER, REVIEW OF SHOP DRAWINGS, ETC., UNLESS THE CONTRACTOR HAS SPECIFICALLY INFORMED THE ENGINEER OF SUCH DEVIATION AT THE TIME OF SUBMISSION, AND THE ENGINEER HAS GIVEN WRITTEN APPROVAL TO THE SPECIFIC DEVIATION.
6. CONSTRUCTION LOADS APPLIED TO THE STRUCTURAL FRAMING SHALL NOT EXCEED THE FINAL DESIGN CRITERIA INDICATED HEREIN FOR ANY OF THE BUILDING MEMBER ELEMENTS.
7. SCALES NOTED ON THE DRAWINGS ARE FOR GENERAL INFORMATION ONLY. NO DIMENSIONS SHALL BE OBTAINED FROM DIRECT SCALING OF THE DRAWINGS.
8. THE CONTRACTOR SHALL COMPARE STRUCTURAL AND ARCHITECTURAL SECTIONS REPORT TO FABRICATION AND/OR INSTALLATION OF STRUCTURAL MEMBERS. REPORT ANY DISCREPANCIES TO THE ARCHITECT/ENGINEER.
9. REFER TO ARCHITECTURAL, MECHANICAL, ELECTRICAL, AND PLUMBING DRAWINGS FOR LOCATION AND SIZES OF OPENINGS AND EQUIPMENT NOT SHOWN ON THE STRUCTURAL DOCUMENTS. THE CONTRACTOR SHALL CONFIRM SIZES AND LOCATIONS PRIOR TO FABRICATION/INSTALLATION OF STRUCTURAL FRAMING.
10. MODIFICATIONS AND/OR ALTERATIONS TO STRUCTURAL MEMBERS, INCLUDING CHANGES IN SIZES AND DIMENSIONS, SHALL NOT BE MADE WITHOUT THE WRITTEN APPROVAL OF THE ENGINEER.

1. THE STRUCTURAL DRAWINGS PROVIDE REPRESENTATIVE DETAILS OF THE FRAMING, AND ARE NOT INTENDED TO SHOW EVERY POSSIBLE CONDITION. SHOP DRAWINGS SHALL DETAIL ALL CONDITIONS IN ACCORDANCE WITH THE SPECIFIC REQUIREMENTS AS INDICATED IN THE PROJECT DOCUMENTS.
2. SHOP DRAWINGS PREPARED SHALL BE COORDINATED, REVIEWED, AND SIGNED/STAMPED BY THE CONTRACTOR PRIOR TO SUBMITTING TO THE ENGINEER FOR APPROVAL.
3. THERE SHALL BE NO DEVIATION FROM THE STRUCTURAL DOCUMENTS. IF ANY CHANGES ARE PROPOSED BY THE CONTRACTOR OR THE PROVIDER OF THE SHOP DRAWINGS, THEY WILL ONLY BE CONSIDERED IF THE FOLLOWING CRITERIA IS MET:
 - A. COST SAVINGS TO THE OWNER
 - B. PRODUCT OR MATERIAL IS AN APPROVED EQUIVALENT WITH THE PROPER CERTIFICATIONS SUBMITTALS NOT MEETING THIS CRITERIA WILL NOT BE CONSIDERED.
4. STRUCTURAL CONTRACT DOCUMENTS SHALL NOT BE REPRODUCED FOR USE AS SHOP DRAWINGS.

1. THE FOUNDATION SYSTEM HAS BEEN DESIGNED FOR A MAXIMUM NET ALLOWABLE SOIL BEARING PRESSURE OF 2500 PSF FOR INDIVIDUAL COLUMN SPREAD FOOTINGS AND 2500 PSF FOR CONTINUOUS WALL FOOTINGS UNDER FULL SERVICE LOADS.
2. FOUNDATION DESIGN COMPLIES WITH THE RECOMMENDATIONS PROVIDED IN THE GEOTECHNICAL REPORT DATE 02/19/2025 BY TERRACON (PROJECT #HN245187). THE CONTRACTOR SHALL PERFORM EXCAVATIONS, SUBGRADE PREPARATION, AND FOOTING CONSTRUCTION IN ACCORDANCE WITH THESE RECOMMENDATIONS, AS WELL AS CIVIL DRAWINGS AND PROJECT SPECIFICATIONS.
3. FOOTINGS SHALL BEAR ON NATURAL UNDISTURBED SOIL OR ENGINEERED FILL. ALL SOIL BELOW SLABS AND FOOTINGS SHALL BE PROPERLY COMPACTED AND SUBGRADE BROUGHT TO A REASONABLE TRUE AND LEVEL PLANE BEFORE CONCRETE PLACEMENT.
4. EXISTING ORGANIC MATERIALS, UNSUITABLE SOILS, ABANDONED FOOTINGS, OR ANY OTHER UNSUITABLE MATERIALS ENCOUNTERED SHALL BE REMOVED AND REPLACED WITH NEW FILL MATERIAL OF SIMILAR TYPE SOIL AS PRESENT AT SITE AND APPROVED BY GEOTECHNICAL ENGINEER. NEW FILL SHALL BE COMPACTED TO A MINIMUM 98% MAXIMUM DRY DENSITY OR AS REQUIRED BY GEOTECHNICAL REPORT. ADEQUATE FIELD DENSITY AND MOISTURE CONTENT TESTS SHALL BE PERFORMED BY AN INDEPENDENT TESTING AGENCY TO ENSURE COMPLIANCE. ALL BEARING SOILS SHALL BE APPROVED BY A GEOTECHNICAL ENGINEER PRIOR TO PLACEMENT OF FOOTINGS.
5. BOTTOM OF ALL EXTERIOR FOOTINGS SHALL BEAR A MINIMUM OF 1'-6" BELOW FINAL GRADE FOR FROST PROTECTION.
6. ELEVATIONS SHOWN ON PLAN INDICATE TOP OF FOOTINGS, BASED ON INFORMATION AVAILABLE AT TIME OF DESIGN. FOOTING ELEVATIONS SHALL BE ADJUSTED AS REQUIRED TO SUIT FIELD CONDITIONS.
7. FOOTINGS MAY BE EARTH-FORMED IF EXCAVATION CAN BE KEPT VERTICAL, CLEAN, AND STABLE. OTHERWISE, FORMS MUST BE USED.
8. FOOTING EXCAVATIONS SHALL HAVE THE SIDES AND BOTTOM TEMPORARILY LINED WITH A PROTECTIVE PLASTIC MEMBRANE IF PLACEMENT OF CONCRETE DOES NOT OCCUR WITHIN 24 HOURS OF THE EXCAVATION OF THE FOOTING. IF THE FOOTING BEARING SURFACE IS ALLOWED TO BECOME DISTURBED IN ANY WAY, IT SHALL BE REWORKED TO THE SATISFACTION OF AN INDEPENDENT TESTING AGENCY PRIOR TO CASTING OF THE CONCRETE.
9. HORIZONTAL CONSTRUCTION JOINTS ARE NOT PERMITTED. FOR VERTICAL CONSTRUCTION JOINTS, PROVIDE LOCATIONS FOR ENGINEER APPROVAL.
10. COLUMN FOOTINGS AND WALL FOOTINGS SHALL BE POURED MONOLITHIC WITH TOPS OF ADJACENT FOOTINGS AT THE SAME ELEVATION (UNO).

1. EXCAVATION, BACKFILLING, AND COMPACTION OPERATIONS SHALL BE EXECUTED IN ACCORDANCE WITH INDUSTRY STANDARDS.
2. FOOTING EXCAVATIONS SHALL BE MADE TO THE PROPER LINE AND LEVEL TO ENSURE ADEQUATE CONCRETE COVER FOR REINFORCEMENT.
3. NO EXCAVATION SHALL BE CLOSER THAN AT A SLOPE OF 2:1 (2 HORIZONTAL TO 1 VERTICAL) TO A FOOTING. PROVIDE TEMPORARY SHORING AND PROTECTION FOR EXCAVATION BANKS AS NECESSARY TO PRESERVE SAFETY AND PREVENT CAVING.
4. BACKFILL SHALL BE PLACED AND COMPACTED, UTILIZING ACCEPTABLE COMPACTION EQUIPMENT, WITH APPROVAL BY A LICENSED GEOTECHNICAL ENGINEER.
5. WALLS ARE NOT DESIGNED TO SUPPORT HYDROSTATIC PRESSURES. THE CONTRACTOR SHALL PROVIDE AN ADEQUATE DRAINAGE SYSTEM FOR ALL BACKFILL CONDITIONS. REFER TO CIVIL AND ARCHITECTURAL DRAWINGS, AND PROJECT SPECIFICATIONS FOR MINIMUM REQUIREMENTS.
6. BACKFILL SHALL NOT BE PLACED AGAINST RETAINING FOUNDATION WALLS UNLESS TOP AND BOTTOM OF WALL IS ADEQUATELY BRACED. AT TOP, STRUCTURE MUST BE SET/INSTALLED TO BE CONSIDERED BRACED. IF THESE ELEMENTS WILL NOT BE IN PLACE AT TIME OF BACKFILLING OPERATIONS, CONTRACTOR SHALL PROVIDE A TEMPORARY BRACING SYSTEM ENGINEERED AND SEALED BY A REGISTERED ENGINEER IN THE PROJECT STATE. BRACING PLANS AND DETAILS ARE TO BE SUBMITTED TO ARCHITECT AND ENGINEER FOR APPROVAL PRIOR TO BACKFILLING.
7. RETAINING WALLS ARE NOT DESIGNED TO CANTILEVER AT ANY TIME, UNLESS EXPLICITLY NOTED ON THE DRAWINGS.

1. ALL CONCRETE DESIGN AND CONSTRUCTION SHALL BE IN ACCORDANCE WITH ACI 318-14 AND ACI 301-10.
2. CEMENT USED SHALL BE TYPE I OR III CONFORMING TO ASTM C-150. CONCRETE SHALL DEVELOP A MINIMUM 28 DAY STRENGTH AND DENSITY AS FOLLOWS:

	STRENGTH (PSI)	DENSITY (PCF)	W/C RATIO
FOOTINGS, PIERS, WALLS	3000	145 - 150	0.5
INTERIOR SLAB ON GRADE	3000	145 - 150	0.5
3. AGGREGATE SHALL BE WELL GRADATED AND SHALL CONFORM TO THE FOLLOWING:

	1" COARSE AGGREGATE (ASTM C-33)
FOOTINGS, PIERS, WALLS, SLAB ON GRADE (DENSITY 145 - 150 PCF)	
4. CONTRACTOR SHALL SUBMIT CONCRETE MIX DESIGNS FOR REVIEW IN ADVANCE OF CONCRETE PLACEMENT. CONCRETE MIX DESIGN SHALL INCLUDE ALL STRENGTH DATA NECESSARY TO SHOW COMPLIANCE WITH THE PROJECT SPECIFICATIONS BY EITHER THE TRIAL BATCH OR FIELD EXPERIENCE METHOD AND SHALL BE CERTIFIED BY A PROFESSIONAL ENGINEER REGISTERED IN THE PROJECT STATE. RESULTS OF ALL COMPRESSIVE STRENGTH TESTS SHALL BE MADE AVAILABLE AT THE JOB SITE FOR REVIEW BY THE INSPECTOR.
5. ALL MIXING, TRANSPORTING, PLACING AND CURING OF CONCRETE SHALL BE DONE IN ACCORDANCE WITH THE RECOMMENDATIONS OF THE AMERICAN CONCRETE INSTITUTE.
6. NO ADDITIONAL WATER SHALL BE ADDED TO CONCRETE AT THE JOB SITE.
7. AT LOCATIONS WHERE NEW CONCRETE IS PLACED ADJACENT TO EXISTING CONCRETE, ROUGHEN AND CLEAN INTERFACE AND APPLY BONDING AGENT (SIKADUR 32 HI-MOD OR APPROVED EQUAL).
8. MINIMUM CONCRETE COVER UNLESS NOTED OTHERWISE:
 - A. INTERIOR CONCRETE NOT EXPOSED TO EARTH OR WEATHER
 - #11 BARS AND SMALLER: 3/4 INCHES
 - B. UNFORMED SURFACE IN CONTACT WITH THE GROUND: 3 INCHES
 - C. BASEMENT WALLS: 2 INCHES EXT
3/4 INCHES INT
 - D. FORMED SURFACES EXPOSED TO EARTH OR WEATHER:
 - #6 BARS AND LARGER: 2 INCHES
 - #5 BARS AND SMALLER: 1 1/2 INCHES
 - E. FORMED SURFACES NOT EXPOSED TO EARTH OR WEATHER:
 - BEAMS, GIRDERS AND COLUMNS: 1 1/2 INCHES
 - SLABS, WALLS, AND JOISTS: 3/4 INCHES
9. SLAB-ON-GRADE SHALL BE SAW CUT IMMEDIATELY AFTER CONCRETE HARDENS. CONTRACTOR TO SUBMIT LAYOUT AND CONSTRUCTION SCHEDULE ("SOFT- CUT" INTERNATIONAL OR SIM.)
10. PROVIDE TEMPORARY SHORING AND BRACING OF ALL STRUCTURAL AND MISCELLANEOUS ELEMENTS UNTIL CONCRETE HAS OBTAINED 80% OF DESIGN STRENGTH AND ALL PERMANENT BRACING ELEMENTS ARE INSTALLED.
11. PLACEMENT OF CONCRETE, COLD WEATHER AND HOT WEATHER PRECAUTIONS, MATERIAL, AND PROPORTIONING REQUIREMENTS, REBAR COVER AND DETAILING SHALL CONFORM TO REQUIREMENTS OF THE AMERICAN CONCRETE INSTITUTE (ACI) 318-11.
12. REFER TO ARCHITECTURAL AND MECHANICAL DRAWINGS AND SPECIFICATIONS FOR SLAB FINISHES, SLAB DEPRESSIONS, ELEVATIONS AND ENCASED OR EMBEDDED ITEMS.
13. PIPES AND CONDUITS EMBEDDED IN CONCRETE SHALL CONFORM TO THE FOLLOWING REQUIREMENTS:
 - A. PIPES AND CONDUITS EMBEDDED IN CONCRETE SHALL CONFORM TO THE FOLLOWING REQUIREMENTS:
 - NO MATERIAL HARMFUL TO CONCRETE (SUCH AS, BUT NOT LIMITED TO, ALUMINUM) IS PERMITTED.
 - B. NO EMBEDMENT OR PENETRATION WHICH IMPAIRS THE STRUCTURAL STRENGTH OR INTEGRITY IS PERMITTED.
 - C. CONDUITS AND PIPES SHALL NOT HAVE A DIAMETER THAT EXCEEDS 1/3 THE OVERALL THICKNESS OF THE STRUCTURAL ELEMENT IN WHICH THEY ARE EMBEDDED.
 - D. MINIMUM CENTER TO CENTER SPACING SHALL NOT BE CLOSER THAN 3 DIAMETERS OR WIDTHS.
 - E. PLACEMENT SHALL OCCUR ABOVE BOTTOM LAYER OF REINFORCEMENT AND BELOW TOP LAYER OF REINFORCEMENT AND SHALL NOT CAUSE REINFORCEMENT TO BE CUT, BENT, OR DISPLACED IN ANY MANNER.
 - F. PLACEMENT SHALL MAINTAIN A MINIMUM CLEARANCE FROM REINFORCEMENT OF 3 REINFORCING BAR DIAMETERS OR 3/4" FROM WELDED WIRE FABRIC REINFORCEMENT.
 - G. PLUMBING AND ELECTRICAL CONDUITS SHALL BE PLACED BELOW SLAB ON GRADE.
14. UNLESS NOTED OTHERWISE, PROVIDE CONSTRUCTION JOINTS IN SLABS ON GRADE AT APPROXIMATELY 50 FEET IN EACH DIRECTION. PROVIDE CONTROL JOINTS IN SLABS ON GRADE NOT TO EXCEED 15 FEET ON CENTER IN EACH DIRECTION, UNLESS OTHERWISE APPROVED BY THE STRUCTURAL ENGINEER.
15. FORMING SHALL BE OF WOOD, STEEL, OR FIBERGLASS OF SATISFACTORY QUALITY AND CONDITION.
16. NO ADMIXTURES SHALL BE ADDED TO THE CONCRETE UNLESS APPROVED BY THE ENGINEER.
17. REINFORCING SHALL CONFORM TO ASTM A615, GR60 UNLESS NOTED OTHERWISE.
18. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A185 GRADE 60.

19. REINFORCING STEEL AND ACCESSORIES SHALL BE DETAILED IN ACCORDANCE WITH ACI 315-99 (MANUAL OF STANDARD PRACTICE FOR DETAILING CONCRETE STRUCTURES) AND CRSI MSP-1 "MANUAL OF STANDARD PRACTICE", LATEST EDITION.
20. ALL "CONTINUOUS" REINFORCEMENT SHALL HAVE MINIMUM LAP OF "B" TYPE (ACI 318-11, SECTION 12.15.1) AT SPLICES UNLESS NOTED OTHERWISE.
21. PROVIDE REINFORCING CHAIRS FOR ALL SLAB-ON-GRADE REINFORCING. WELDED WIRE FABRIC REINFORCING SHALL NOT BE "PULLED UP" DURING PLACEMENT.
22. SUBMIT REINFORCING PLACEMENT AND DETAIL (SHOP) DRAWINGS FOR REVIEW. NO REINFORCING BARS SHALL BE INSTALLED UNTIL THE SHOP DRAWINGS HAVE BEEN REVIEWED AND RETURNED.
23. ALL REINFORCING SHALL BE SUPPORTED IN FORMS SPACED WITH NECESSARY ACCESSORIES AND SHALL BE SECURELY WIRED TOGETHER IN ACCORDANCE WITH CRSI "MANUAL OF STANDARD PRACTICE", LATEST EDITION.
24. WHERE WELDED WIRE FABRIC REINFORCEMENT IS SPECIFIED IN SLABS ON GRADE PLACEMENT SHALL BE 1" BELOW TOP OF SLAB FOR 4" THICK SLABS OR 2" BELOW TOP OF SLAB FOR 6" THICK SLABS. OVERLAP EACH REINFORCING SHEET TWO FULL PANELS AND TIE CROSS WIRES ON EACH SIDE.
25. SCHEDULED OR DETAILED REINFORCING STEEL SHALL NOT BE TACK WELDED FOR ANY REASON. WELDED REINFORCING STEEL AND/OR SPLICES ARE PERMITTED ONLY WHERE SHOWN ON DRAWINGS. WHERE WELDING IS PERMITTED IT SHALL CONFORM TO AWS D1.4, STRUCTURAL WELDING CODE - REINFORCING STEEL.
26. BASE PLATES, ANCHOR RODS, SUPPORT ANGLES, ETC. BELOW GRADE SHALL BE COVERED WITH A MINIMUM OF 4" OF CONCRETE.
27. WHERE FOOTINGS, WALLS, OR OTHER STRUCTURAL ELEMENTS INTERSECT, CORNER OR TEE, PROVIDE CORNER BARS WITH REQUIRED LAP LENGTHS TO PROVIDE CONTINUITY OF HORIZONTAL STEEL REINFORCING UNLESS NOTED OTHERWISE.
28. WHERE DOWELS, BOLTS OR INSERTS ARE CALLED TO BE ANCHORED TO CAST IN PLACE OR PRECAST CONCRETE ELEMENTS USING EPOXY ADHESIVES, USE ANCHORAGE SYSTEM EQUAL TO HILTI HIT DOWELING (HY-200 MAX). FOLLOW ALL MANUFACTURER'S RECOMMENDATIONS. ALTERNATE ANCHORAGE SYSTEMS MAY BE PERMITTED WITH STRUCTURAL ENGINEER'S APPROVAL.
29. CONTRACTOR SHALL INCLUDE IN THE BID 4 CU YARDS OF UNSCHEDULED CONCRETE (3,000 PSI) TO BE POURED AS DIRECTED BY THE ENGINEER.

THOMAS E. MORGAN, JR.
ARCHITECT

423 FISCHER TRAIL
ELLIJAY, GEORGIA 30541

A circular professional engineer seal for the State of Oregon. The outer ring contains the text "STATE OF OREGON" at the top and "WALDEMAR E DEL VALLE" at the bottom, separated by two stars. Inside the ring, the word "ENGINEER" is written in a semi-circle. In the center, the text "No. SE000299" and "STRUCTURAL" are displayed. A signature is written across the seal.

6/29/2026



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RELEASE
FOR CONSTRUCTION

[illegible]

DRAWN BY: CB

CHKD BY: MN

PROJECT DESCRIPTION

PROJECT TITLE

TRAVEL CENTER
5524 GA HIGHWAY 96 W
JEFFERSONVILLE
GEORGIA 31044

SHEET TITLE

GENERAL NOTES

PROJECT NO:

S001

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POST-INSTALLED ANCHORS

1. POST-INSTALLED ANCHORS SHALL INCLUDE ADHESIVE AND MECHANICAL ANCHORS.
2. POST-INSTALLED ANCHORS SHALL ONLY BE USED WHERE SPECIFIED ON THE DRAWINGS. CONTRACTOR SHALL OBTAIN APPROVAL FROM ENGINEER OF RECORD PRIOR TO USING POST-INSTALLED ANCHORS FOR MISSING OR MISPLACED CAST-IN-PLACE ANCHORS.
3. CARE SHALL BE GIVEN TO AVOID CONFLICTS WITH EXISTING REBAR WHEN DRILLING HOLES. HOLES SHALL BE DRILLED AND CLEANED PER THE MANUFACTURER'S INSTRUCTIONS.
4. ANCHORS SHALL BE PER MANUFACTURER'S INSTALLATION INSTRUCTIONS AT NOT LESS THAN MINIMUM EDGE DISTANCES AND/OR SPACINGS INDICATED IN THE MANUFACTURER'S LITERATURE.
5. ADHESIVE ANCHORS MUST BE INSTALLED IN CONCRETE AGED A MINIMUM OF 21 DAYS PER ACI 318, APPENDIX D. MANUFACTURER APPROVAL LETTER WILL BE REQUIRED FOR EARLIER INSTALLATION TIMES. NOTIFY EOR FOR FINAL APPROVAL.
6. ALL ADHESIVE ANCHOR INSTALLATIONS IN THE HORIZONTAL TO VERTICALLY OVERHEAD ORIENTATION SHALL BE CONDUCTED BY A CERTIFIED ADHESIVE ANCHOR INSTALLER AS CERTIFIED BY ACI/CSRI PER ACI 318. CURRENT AAI CERTIFICATES MUST BE SUBMITTED TO THE ENGINEER OF RECORD FOR APPROVAL PRIOR TO COMMENCEMENT OF ANY ADHESIVE ANCHOR INSTALLATIONS.
7. ALL POST-INSTALLED ANCHORS IN CONCRETE SHALL HAVE CURRENT PUBLISHED ICC-ES EVALUATION REPORT INDICATING THE ANCHOR IS APPROVED FOR INSTALLATION IN CRACKED CONCRETE AND SHALL HAVE SEISMIC QUALIFICATION TO MEET THE PROJECT REQUIREMENTS.
8. SPECIAL INSPECTION SHALL BE PROVIDED FOR ALL ADHESIVE AND MECHANICAL ANCHORS. SPECIAL INSPECTOR SHALL PERFORM INSPECTIONS PURSUANT TO CONTRACT DOCUMENTS, SCHEDULE OF SPECIAL INSPECTIONS AND APPROPRIATE ICC-ES REPORTS.
9. SUBSTITUTION REQUESTS, FOR PRODUCTS OTHER THAN THOSE LISTED BELOW, SHALL BE SUBMITTED TO THE ENGINEER FOR ACCEPTANCE. PRODUCT ICC-ES CODE REPORTS SHALL BE INCLUDED WITH SUBMITTAL PACKAGE.
10. UNLESS NOTED OTHERWISE ON PLANS, ACCEPTABLE PRODUCTS SHALL BE:

A. ADHESIVE ANCHORS

I. FOR ANCHORING INTO CONCRETE

- SIMPSON STRONG-TIE: ACRYLIC-TIE WITH ASTM A36 THREADED RODS

- HILTI: HIT-HY 200 WITH STANDARD HAS-E THREADED RODS

- DEWALT/POWERS: AC200+ WITH ASTM A36 THREADED RODS

II. FOR ANCHORING INTO GROUT-FILLED CMU

- SIMPSON STRONG-TIE: ACRYLIC-TIE WITH ASTM A36 THREADED RODS

- HILTI: HIT-HY 70 WITH STANDARD HAS-E THREADED RODS

- DEWALT/POWERS: AC100+GOLD WITH ASTM A36 THREADED RODS

III. FOR ANCHORING INTO HOLLOW BASE MATERIAL (HOLLOW CMU)

- SIMPSON STRONG-TIE: ACRYLIC-TIE AND METAL SCREEN TUBES WITH ASTM A36 THREADED RODS

- HILTI: HIT-HY 70 AND METAL SCREEN TUBES WITH HAS-E THREADED RODS

- DEWALT/POWERS: AC100+GOLD AND METAL SCREEN TUBES WITH ASTM A36 THREADED RODS

B. MECHANICAL ANCHORS

I. FOR ANCHORING INTO CONCRETE

- SIMPSON STRONG-TIE: TITEN HD (SCREW ANCHOR) OR STRONG-BOLT 2 (WEDGE ANCHOR)

- HILTI: HUS-EZ (SCREW ANCHOR) OR KWIK BOLT-TZ (WEDGE ANCHOR)

- DEWALT/POWERS: POWER-STUD SD1+ (WEDGE ANCHOR) OR SCREW-BOLT+ (SCREW ANCHOR)

II. FOR ANCHORING INTO GROUT-FILLED CMU

- SIMPSON STRONG-TIE: TITEN HD (SCREW ANCHOR) OR STRONG-BOLT 2 (WEDGE ANCHOR)

- HILTI: HUS-EZ (SCREW ANCHOR) OR KWIK BOLT 3 (WEDGE ANCHOR)

- DEWALT/POWERS: POWER-STUD SD1+ (WEDGE ANCHOR) OR SCREW-BOLT+ (SCREW ANCHOR)

III. FOR ANCHORING INTO HOLLOW BASE MATERIAL (HOLLOW CMU)

- ONLY ADHESIVE ANCHORS ARE APPROVED (REFER TO SECTION A-III FOR SUITABLE PRODUCTS)
11. CONTACT ANCHOR MANUFACTURER FOR PRODUCT RELATED QUESTIONS AND AVAILABILITY.

- SIMPSON STRONG-TIE (800) 999-5099

- HILTI (800) 879-8000

- DEWALT/POWERS (800) 524-3244

MASONRY

1. ALL MASONRY DESIGN AND CONSTRUCTION SHALL CONFORM TO ACI 530-13, ASCE 5-13, TMS 402-13 AND ACI 530.1-13, ASCE 6-13, TMS 602-13.
2. MASONRY SHALL BE MEDIUM WEIGHT AND HAVE A MINIMUM COMPRESSIVE STRENGTH, f'm, OF 1500 PSI BASED ON NET CROSS SECTIONAL AREA. MORTAR SHALL CONFORM TO ASTM C270 TYPE S OR M. GROUT SHALL CONFORM TO ASTM C476, WITH A MAXIMUM AGGREGATE SIZE OF 3/8" AND A MINIMUM COMPRESSIVE STRENGTH OF 2000 PSI.
3. REINFORCING BARS SHALL CONFORM TO ASTM A 615 GRADE 60 UNLESS NOTED OTHERWISE.
4. CONTINUOUS WIRE REINFORCING (JOINT REINFORCING) SHALL BE GALVANIZED LADDER TYPE FABRICATED UNITS WITH A SINGLE PAIR OF 9 GAGE SIDE RODS AND 9 GAGE CROSS RODS FABRICATED FROM COLD DRAWN STEEL WIRE COMPLYING WITH ASTM A82. JOINT REINFORCING SHALL BE SPACED AT 16" OC VERTICALLY IN ALL MASONRY WALLS UNLESS NOTED OTHERWISE.
5. VERTICAL CONTROL JOINTS IN MASONRY WALLS ARE NOT INDICATED ON THESE DRAWINGS. HORIZONTAL BOND BEAM AND LINTEL REINFORCING SHALL BE CONTINUOUS ACROSS VERTICAL CONTROL JOINTS. HORIZONTAL JOINT REINFORCING (DUR-O-WALL) SHALL BE TERMINATED ON EITHER SIDE OF VERTICAL CONTROL JOINTS. WALLS SHORTER THAN 15'-0" IN LENGTH SHALL NOT HAVE VERTICAL CONTROL JOINTS.

A. AT EXTERIOR WALLS, SEE ARCHITECTURAL DRAWINGS FOR LOCATIONS OF VERTICAL CONTROL JOINTS. JOINTS SHALL BE PLACED AT A SPACING NOT TO EXCEED 30'-0" ON CENTER. JOINTS SHALL NOT BE LOCATED CLOSER THAN 2'-8" TO THE JAMB OF ANY EXTERIOR WALL OPENING (UNO). JOINTS SHALL NOT BE LOCATED FURTHER THAN 15'-0" FROM ANY CORNER, NOR CLOSER THAN 4'-0" FROM ANY CORNER.

B. AT INTERIOR SHEAR WALLS, JOINTS SHALL BE PLACED AT A SPACING NOT TO EXCEED 30'-0" ON CENTER. JOINTS SHALL NOT BE LOCATED CLOSER THAN 2'-8" TO THE JAMB OF ANY SHEAR WALL OPENING (UNO). JOINTS SHALL NOT BE LOCATED FURTHER THAN 15'-0" FROM ANY CORNER, NOR CLOSER THAN 5'-0" FROM ANY CORNER.

C. AT INTERIOR NON-SHEAR WALLS, VERTICAL CONTROL JOINTS SHALL BE PLACED AT A SPACING NOT TO EXCEED 30'-0" ON CENTER. JOINTS SHALL BE LOCATED AT WALL JAMBS, WHERE PRACTICAL, AND SHALL STEP 8" HORIZONTALLY AT MASONRY LINTEL LOCATIONS. WHERE WALLS SIT ON TOP OF A CAST SLAB-ON-GRADE, ALIGN WALL CONTROL JOINTS WITH SLAB CONTROL JOINTS. JOINTS SHALL BE LOCATED AT ALL CORNER/TEE INTERSECTIONS WHERE THE LEGS OF EACH CORNER/TEE EXCEED 15'-0" IN LENGTH.
6. CONTROL JOINTS SHALL BE LOCATED IN THE INTERIOR WALLS OF THE BUILDING AT A SPACING NOT EXCEEDING 30 FEET. JOINTS SHALL, AT A MINIMUM, BE LOCATED AT INTERSECTING WALLS.
7. MASONRY PILASTERS SHALL BE LOCATED ADJACENT TO CONTROL OR EXPANSION JOINTS.
8. ALL REINFORCED CELLS AND ALL CELLS BELOW FINISH FLOOR SHALL BE GROUTED SOLID.
9. WHEN A FOUNDATION DOWEL DOES NOT LINE UP WITH A VERTICAL BLOCK CORE, IT SHALL NOT BE SLOPED MORE THAN ONE HORIZONTAL IN SIX VERTICAL. DOWELS MAY BE GROUTED INTO A CELL IN VERTICAL ALIGNMENT EVEN THOUGH IT IS IN AN ADJACENT CELL TO THE VERTICAL WALL REINFORCING.
10. REINFORCING STEEL SHALL BE SECURED IN PLACE BEFORE GROUTING STARTS.
11. VERTICAL BARS SHALL BE HELD IN POSITION WITH PRE-MANUFACTURED TIES AT TOP AND BOTTOM AND AT INTERVALS NOT EXCEEDING 200 DIAMETERS OF THE REINFORCING NOR 10 FEET.
12. VERTICAL CELLS THAT WILL BE GROUTED SHALL HAVE A VERTICAL ALIGNMENT TO MAINTAIN A CONTINUOUS UNOBSTRUCTED CELL AREA NOT LESS THAN 2 1/2" x 3".
13. GROUTING SHALL BE STOPPED 1 1/2" BELOW THE TOP OF A COURSE SO AS TO FORM A KEY AT THE POUR JOINT.
14. GROUTING SHALL BE CONSOLIDATED USING A MECHANICAL VIBRATOR PER REQUIREMENTS STATED IN ACI 530-05.
15. GROUTING OF MASONRY BEAMS OVER OPENINGS SHALL BE DONE IN ONE CONTINUOUS OPERATION.
16. ALL BOLTS INSERTED IN THE WALLS SHALL BE GROUTED SOLIDLY INTO POSITION.
17. WHERE EXPANSION BOLTS OR OTHER ANCHORS ARE EMBEDDED INTO THE SIDE OF MASONRY WALLS, THE CELLS SHALL BE FULLY GROUTED AT LEAST 8" ABOVE AND BELOW EACH BOLT OR ANCHOR.
18. WHERE NOT OTHERWISE SHOWN, MASONRY WALL FOOTINGS SHALL BE 12" THICK AND HAVE A MINIMUM OF 4" PROJECTION ON EACH SIDE OF WALL. REINFORCE WITH (2) #5 BARS CONTINUOUS TOP AND BOTTOM.
19. WALLS SHALL BE GROUTED USING LOW LIFT GROUTING TECHNIQUES.
20. ALL MASONRY WALLS SHALL BE ASSUMED TO BE RUNNING BOND, UNLESS NOTED OTHERWISE IN PLAN OR SECTION.

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CHKD BY: MM

PROJECT DESCRIPTION:

PROJECT TITLE:

TRAVEL CENTER
5524 GA HIGHWAY 96 W
JEFFERSONVILLE
GEORGIA 31044

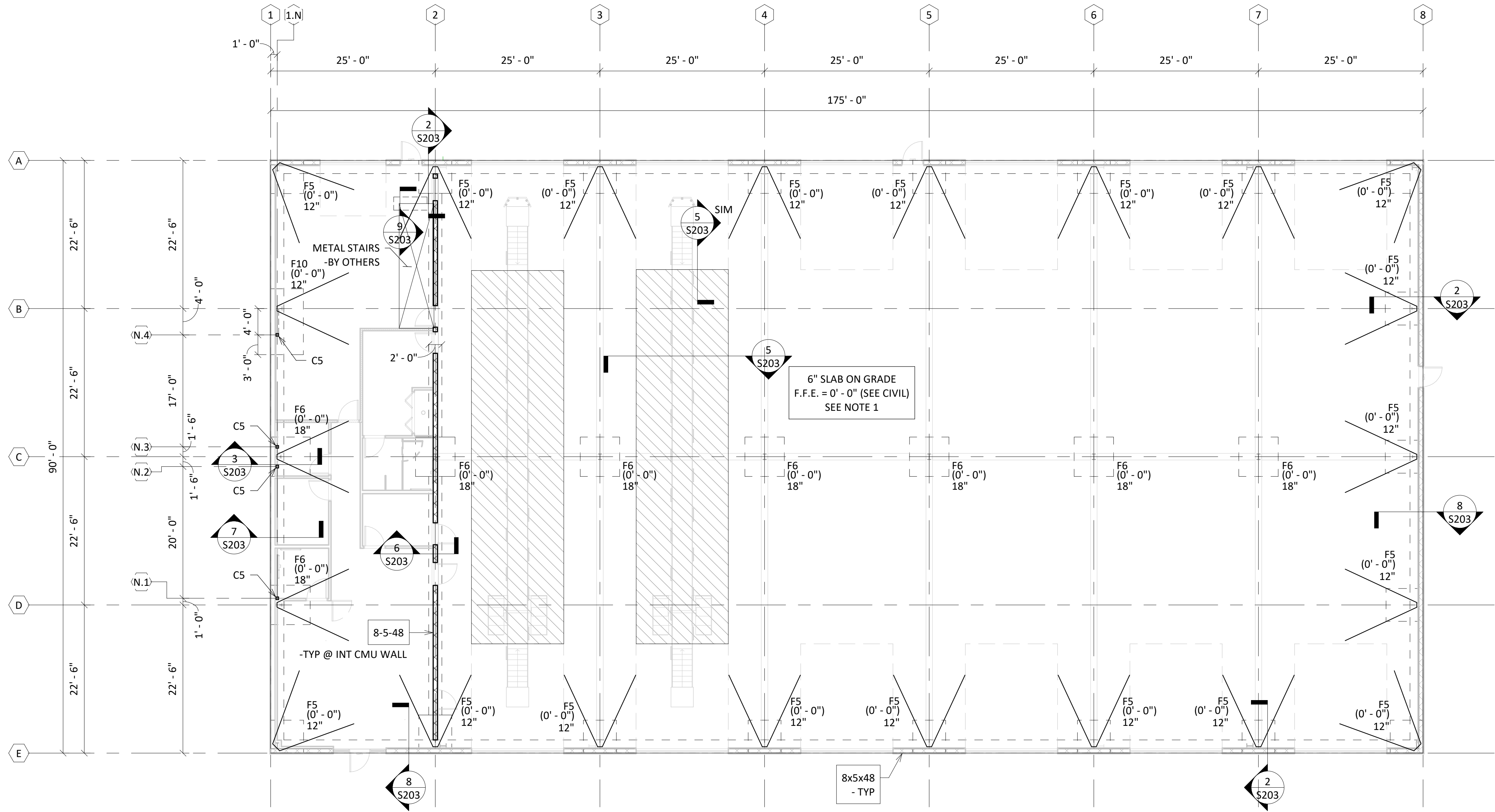
SHEET TITLE:

GENERAL
NOTES

PROJECT NO: 25021

S002

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FOUNDATION NOTES

- SLAB ON GRADE SHALL BE PLACED ON COMPACTED SUB-GRADE & SHALL BE REINFORCED WITH 6x6-W2.9xW2.9 WWF IN FLAT SHEETS (ROLLS NOT PERMITTED). SUPPORT MESH WITH PRECAST CONCRETE SUPPORTS @ 4'-0" OC EACH WAY, 1" CLEAR FROM TOP OF SLAB. LAP MESH 2 SQUARES AT SPLICES, STAGGER SPLICES. PROVIDE VAPOR RETARDER BENEATH FLOOR SLABS WITH JOINTS LAPPED NOT LESS THAN 6" AND TAPED.
- FOUNDATION DESIGN IS BASED ON LOADS PROVIDED BY BUTLER MANUFACTURING (DATED 04/06/2026). LOADS AND FOUNDATIONS ARE SUBJECT TO REVISION IF/WHEN PRE-MANUFACTURED METAL BUILDING SUPPLIER MODIFIES THE BUILDING.
- FOUNDATIONS HAVE BEEN DESIGNED FOR AN ALLOWABLE SOIL BEARING CAPACITY OF 2.5 KSF FOR INDIVIDUAL COLUMN FOOTINGS AND CONTINUOUS WALL FOOTINGS UNDER FULL SERVICE LIVE AND DEAD LOAD AS INDICATED IN THE GEOTECHNICAL REPORT.
- THE FOOTINGS HAVE BEEN POSITIONED AT THE ESTIMATED ELEVATION WHICH WILL PROVIDE SUITABLE BEARING. HOWEVER, IF ADEQUATE BEARING CAPACITY IS NON-EXISTENT AT THESE ESTIMATED ELEVATIONS, THE FOOTING SHALL BE LOWERED TO AN ELEVATION WHERE THE PRESCRIBED SAFE BEARING CAPACITY EXISTS.
- SEE PLAN FOR ALL TOP OF FOOTING ELEVATIONS.
- FOOTINGS SHALL BE LOCATED ON CENTER LINE OF WALL AND/OR COLUMNS - UNO.
- ALL COLUMNS TO BE LOCATED ON CENTER LINE OF WALL - UNO. ORIENT LONG SIDE OF COLUMN IN THE DIRECTION OF ARROW INDICATED.
- PLACE CONTROL JOINTS AT COLUMN LINES & INTERMEDIATED LINES SUCH THAT AREA OF EACH PANEL DOES NOT EXCEED 225 SQUARE FEET. LOCATE CONSTRUCTION JOINTS AT CONTROL JOINTS. AVOID LOCATING CONTROL JOINTS IN THICKENED SLABS UNDER LOAD BEARING WALLS. SEE 10 / S201.
- REFER TO 1 / S201 AND GEOTECHNICAL REPORT FOR ADDITIONAL SUB-GRADE REQUIREMENTS.
- REFER TO ARCH'L AND CIVIL DRAWINGS FOR LOCATIONS OF MOISTURE BARRIER, CURBS, EXTERIOR SLABS, DRAINAGE, RAMP, STEPS, WALKS, ETC.
- BUILDING SLAB IS NOT DESIGNED TO SUPPORT CRANE LOADS, CONCRETE MIXING TRUCKS, OR OTHER SPECIFIC CONSTRUCTION LOADINGS.
- REFERENCE PRE-ENGINEERED METAL BUILDING ANCHOR BOLTS LAYOUT DRAWINGS FOR ANCHOR BOLTS DIAMETER. USE 1/4"x3"x3" PLATE WASHER. SEE METAL BUILDING DRAWINGS FOR NUMBER, LOCATION, AND LAYOUT OF ANCHOR BOLTS. POST-INSTALLED ANCHORS ARE NOT ALLOWED. (ONLY APPLICABLE AT PEMB COLUMNS. REFERENCE SCHEDULES AND DETAILS AT DV+M COLUMNS SHOWN ON PLAN).
- COORDINATE LOCATION OF LOWERED FOOTINGS WITH M/E/P DRAWINGS.

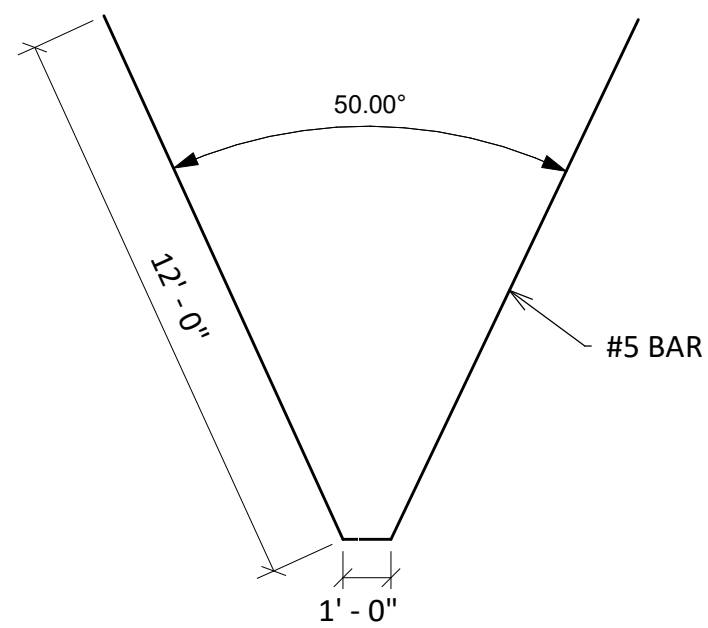
'FX'
(X'-XX")
(XX")

INDICATES FOOTING TAG (SEE SCHEDULE)
INDICATES TOP OF FOOTING ELEVATION
INDICATES ANCHOR BOLT EMBEDMENT AT PEMB COLUMNS ONLY. (REFER TO 5 / S201 FOR INFORMATION PERTAINING TO DV+M COLUMNS SHOWN ON PLAN.)

(X'-XX")

INDICATES TOP OF FOOTING ELEVATION
(AS REFERENCED FROM FINISHED FLOOR ELEVATION)

- IN ADDITION TO SECTIONS AND DETAILS REFERENCED IN THIS PLAN, SEE TYPICAL DETAILS.
- INDICATES STEP IN FOOTING (AS REFERENCED FROM FINISHED FLOOR ELEVATION) - SEE 4 / S201
- PROVIDE A CONCRETE PLASTER FOR TOP OF FOOTING ELEVATIONS LESS THAN -2'-0" A.F.F. SEE SCHEDULE
- INDICATES AREA OF SLAB TO BE RECESSED. COORDINATE EXTENTS WITH ARCH. - SEE 11 / S202
- NOMINAL WALL SIZE (INCHES)
BAR SIZE
BAR SPACING (INCHES)
- INDICATES CMU WALL REINFORCEMENT. ALL REINFORCED CELLS SHALL BE GROUTED SOLID. ALL MASONRY WALLS HAVE LADDER TYPE REINFORCEMENT (MINIMUM NO.9 SIDE RODS) AT 16" OC (UNO).



TYPICAL HAIRPIN DETAIL
SEE PLAN FOR LOCATIONS AND ORIENTATION

STRUCTURAL FOUNDATION SCHEDULE		
FOOTING TAG	FOOTING SIZE	FOOTING REINF
F5	5'-0" x 5'-0" x 24"	(5) #5 E.W. T&B
F6	6'-0" x 6'-0" x 24"	(6) #5 E.W. T&B
F10	10'-0" x 5'-0" x 24"	(10) #5 L.W. T&B & (5) #5 S.W. T&B

DV+M COLUMN SCHEDULE			
COLUMN TAG	COLUMN SIZE	BASE PLATE SIZE	ANCHOR BOLTS
C5	HSS5X5X1/4	11"x11"x3/4"	(4) 3/4" DIA. x 6" EPOXY ANCHORS

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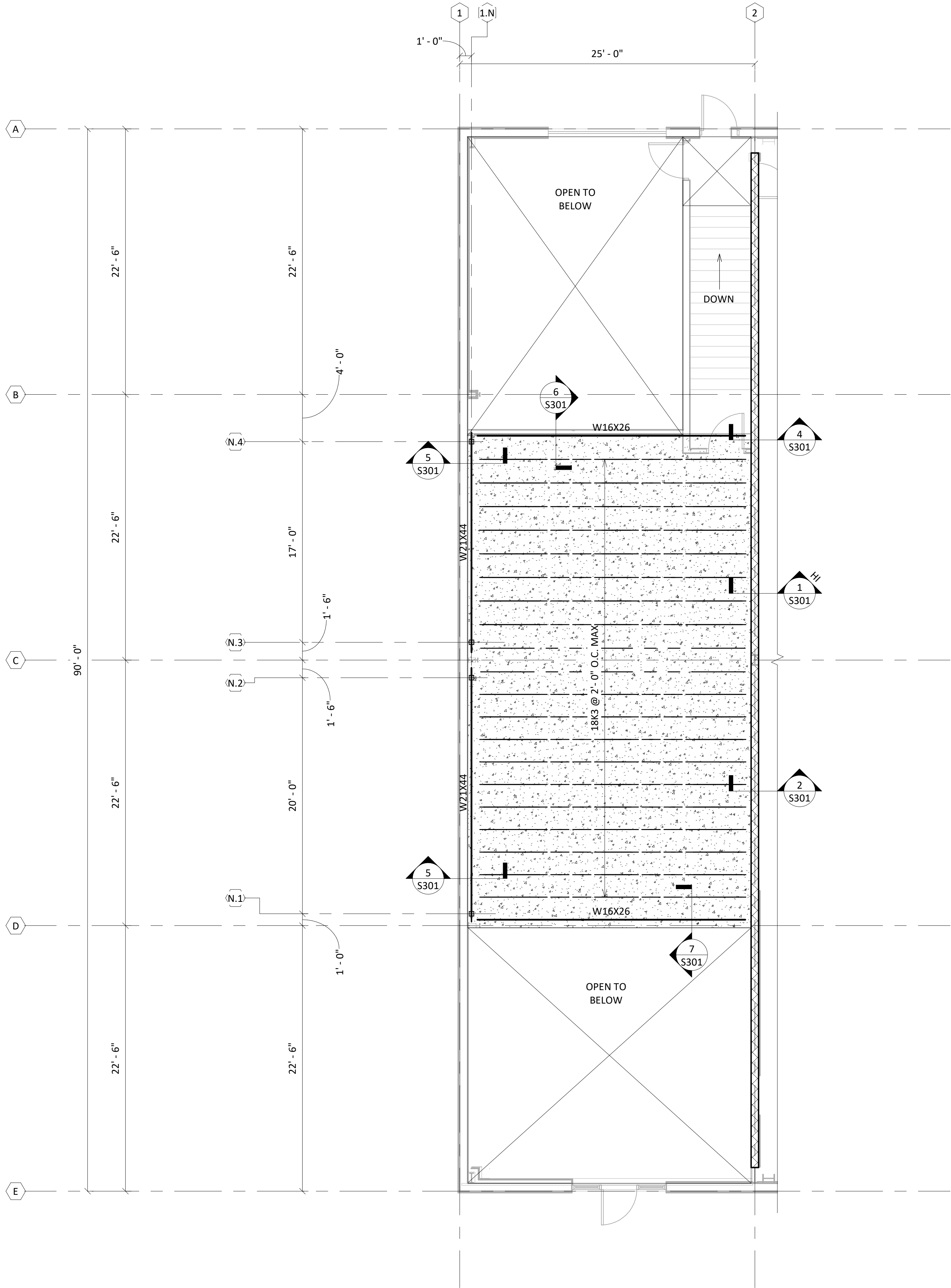
SHEET TITLE:

FOUNDATION
PLAN

PROJECT NO: 25021

S101

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- FLOOR FRAMING NOTES**
- SLAB TO BE 2-1/2" THICK LIGHT WEIGHT CONCRETE (115 PCF AT 3,500 PSI) STRUCTURAL FLOOR SLAB WITH 6x6-W2.1xW2.1 WWF ON 1-1/2", 22 GAGE GALVANIZED COMPOSITE METAL DECK (VULCRAFT 1.5VLI OR EQUAL). TOTAL SLAB THICKNESS = 4".
 - TOP OF SLAB ELEVATION = 10'-5 1/2"
TOP OF STEEL ELEVATION = 10'-1 1/2"
 - CONTRACTOR TO VERIFY MECHANICAL CHASE SIZES AND LOCATIONS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS/REQUIREMENTS PRIOR TO STEEL FABRICATION. NOTIFY ARCHITECT/ENGINEER OF ANY CONFLICTS OR DISCREPANCIES.

FLOOR DESIGN LOADS:
DEAD LOAD: 60.0 PSF
LIVE LOAD: 125.0 PSF

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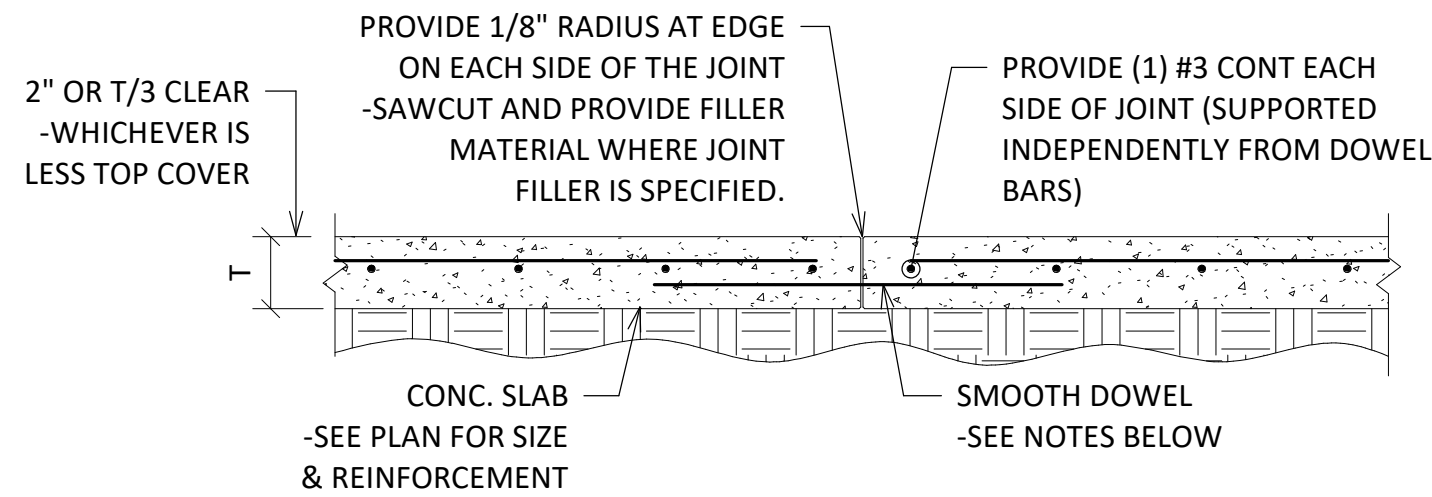
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MEZZANINE
FRAMING
PLAN

PROJECT NO: 25021

S102

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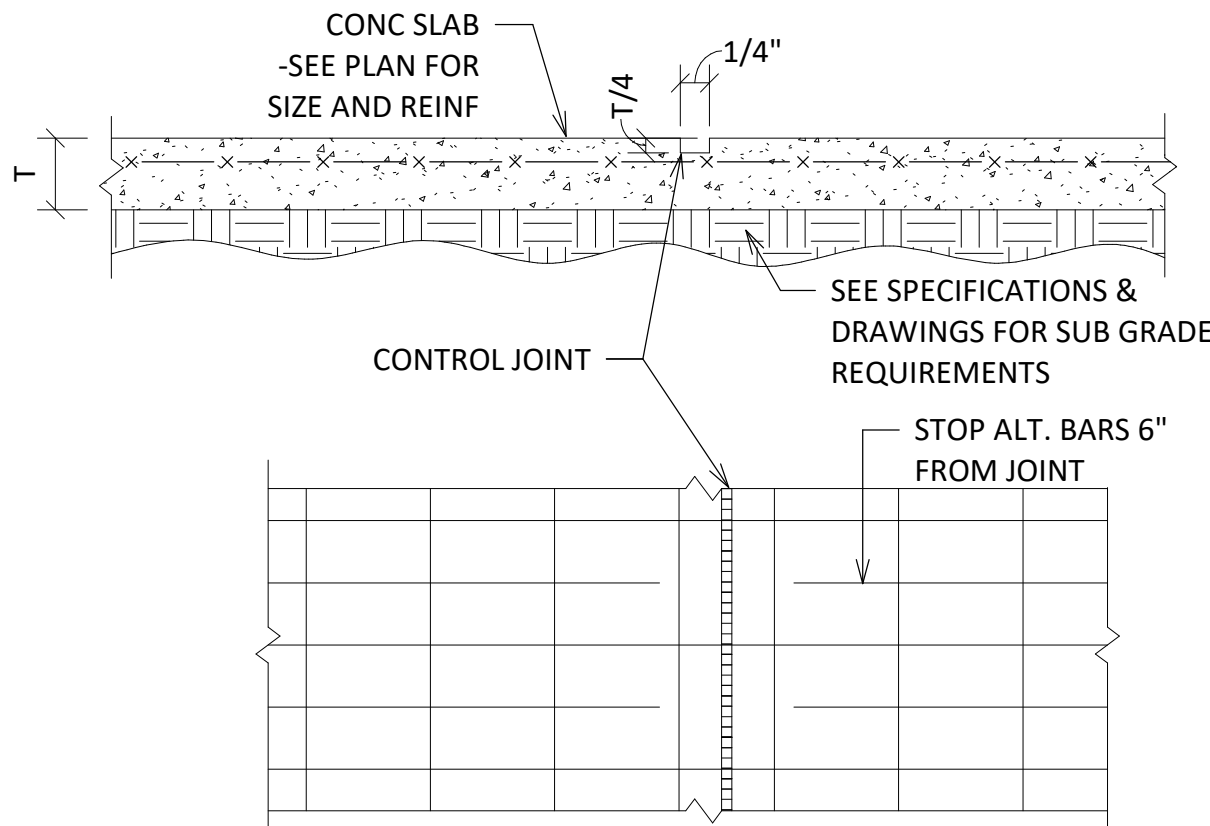


SLAB REINFORCEMENT SHALL BEAR ON SOIL-SUPPORTED SLAB BOLSTER. BREAK BOND BETWEEN NEW AND PREVIOUSLY-PLACED SLAB BY SPRAYING OR PAINTING EXPOSED SIDE OF JOINT WITH A CURING COMPOUND, BONDBREAKER, OR FORM OIL.

PLAN BAR DOWEL SCHEDULE			
SLAB THICKNESS (T)	DOWEL SIZE	DOWEL LENGTH	DOWEL SPACING
$T \leq 5"$	5/8" DIA	24"	16" OC
$5" < T \leq 6\text{-}1/2"$	3/4" DIA	24"	16" OC
$6\text{-}1/2" < T \leq 8"$	3/4" DIA	24"	12" OC

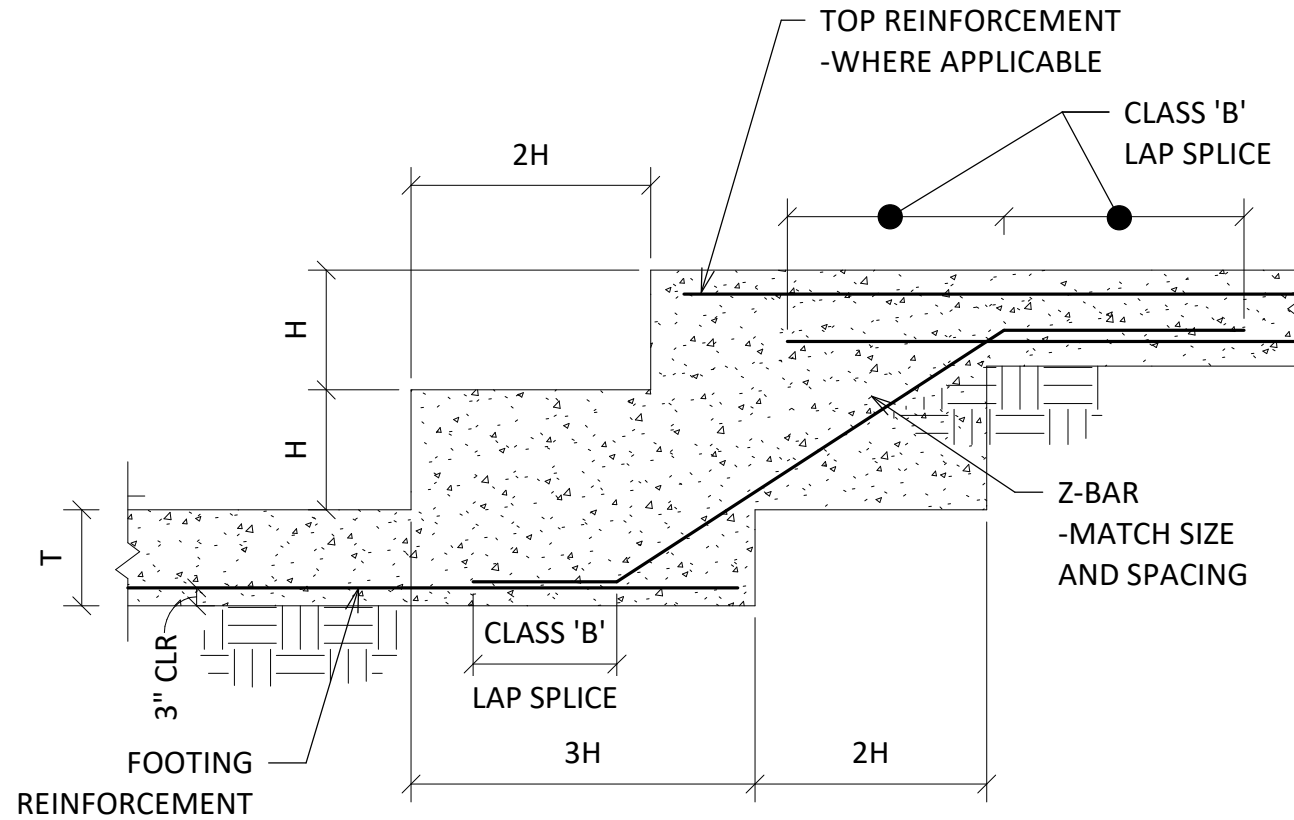
DOWEL NOTES

LIGHTLY COAT THE EXPOSED END OF THE DOWELS WITH A PARAFFIN-BASED LUBRICANT, ASPHALT EMULSION, FORM OIL OR GREASE IMMEDIATELY BEFORE PLACING CONCRETE ON THE SECOND SIDE OF THE JOINT OR USE A PLASTIC OR METAL SLEEVE SPECIFICALLY MANUFACTURED FOR THIS PURPOSE, TO PREVENT A BOND BETWEEN THE DOWEL AND THE CONCRETE.



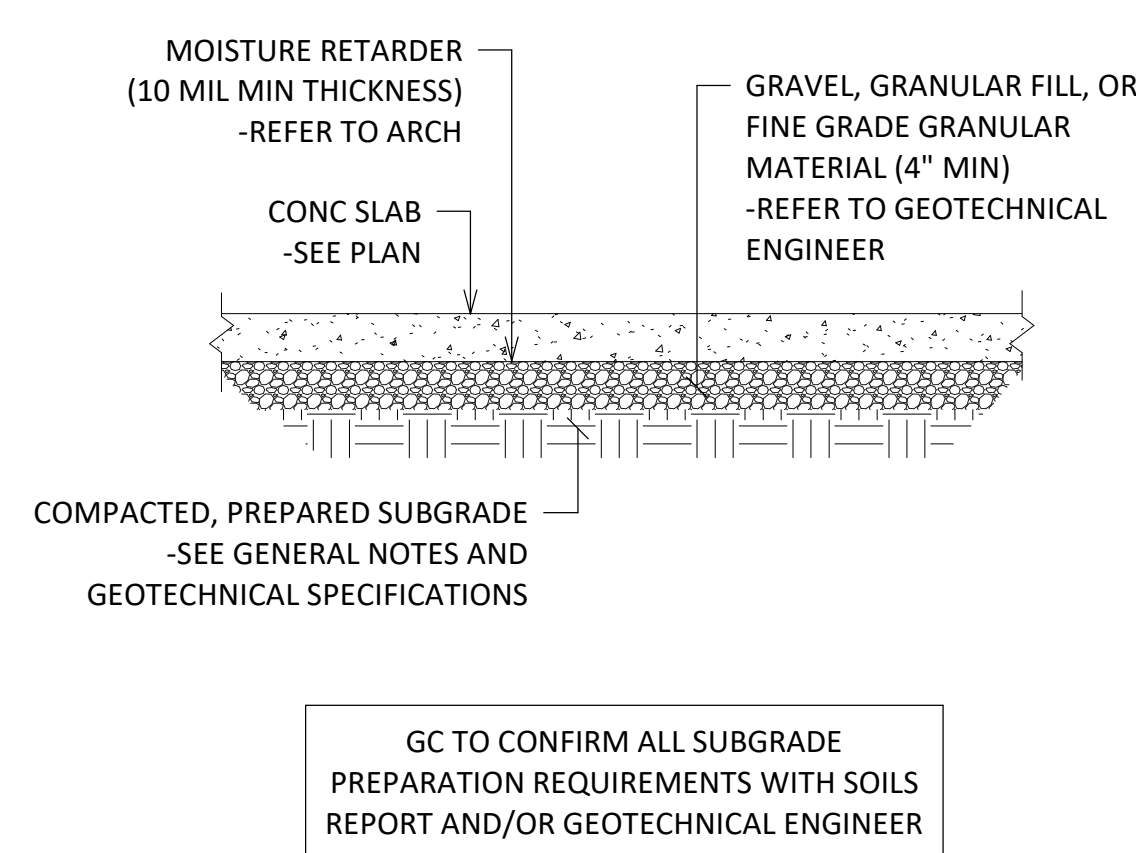
NOTES

1. MAKE SAW CUT AS SOON AS SLAB IS ABLE TO SUPPORT WEIGHT OF WORKERS AND SAWING EQUIPMENT WITHOUT DAMAGE TO FINISH SURFACE OF SLAB. PROVIDE CONTRACTION OR CONSTRUCTION JOINT AT EVERY COLUMN LINE AND IN BETWEEN ADJACENT COLUMN LINE SUCH THAT THE JOINT SPACING DOES NOT EXCEED 36 TIMES THE SLAB THICKNESS (SLAB THICKNESS GREATER THAN OR EQUAL TO 4 IN) OR 12 FEET, WHICHEVER IS LESS.



NOTES

1. MAXIMUM VERTICAL STEP IS 2'-0"
2. SEE PLAN FOR FOOTING STEP ELEVATIONS



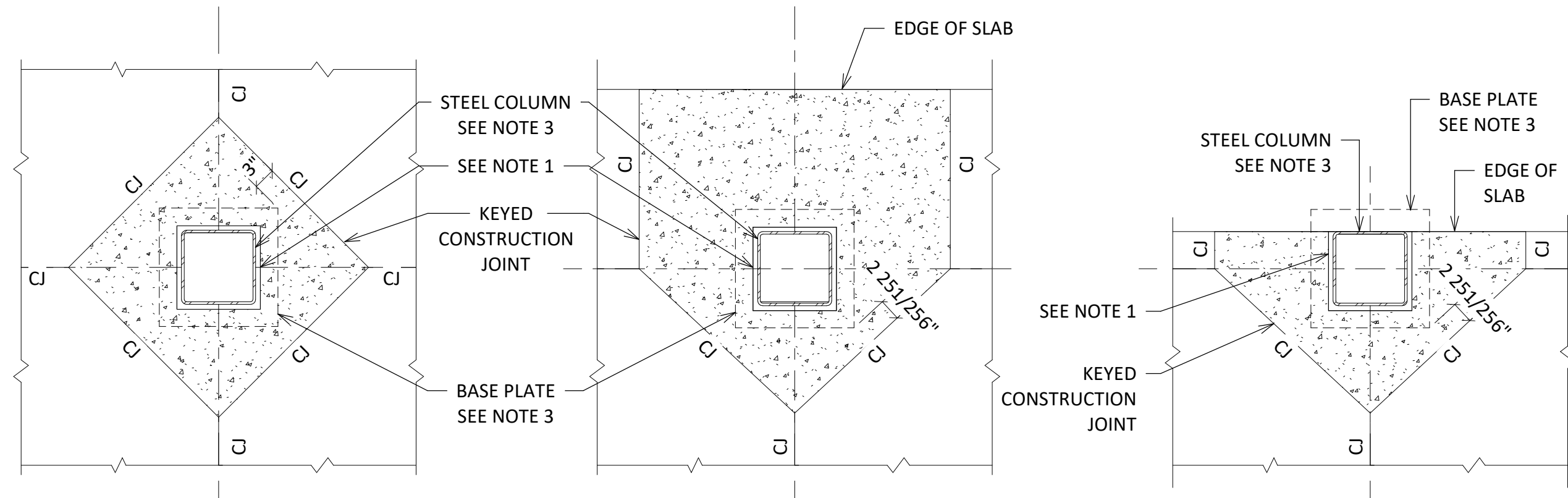
GC TO CONFIRM ALL SUBGRADE PREPARATION REQUIREMENTS WITH SOILS REPORT AND/OR GEOTECHNICAL ENGINEER

10 SLAB-ON-GRADE CONSTRUCTION JOINTS AND REINFORCEMENT DETAIL
3/4" = 1'-0"

7 SLAB-ON-GRADE CONTROL JOINTS AND REINFORCEMENT DETAIL
3/4" = 1'-0"

4 STEPPED FOOTING DETAIL
3/4" = 1'-0"

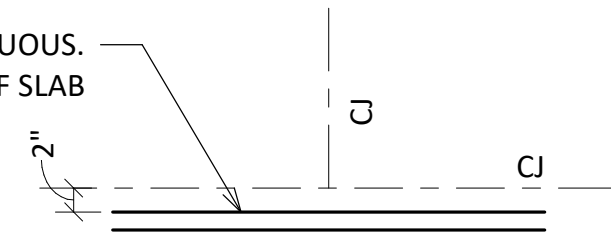
1 SLAB ON GRADE SUBGRADE PREPARATION
3/4" = 1'-0" TYPICAL DETAIL



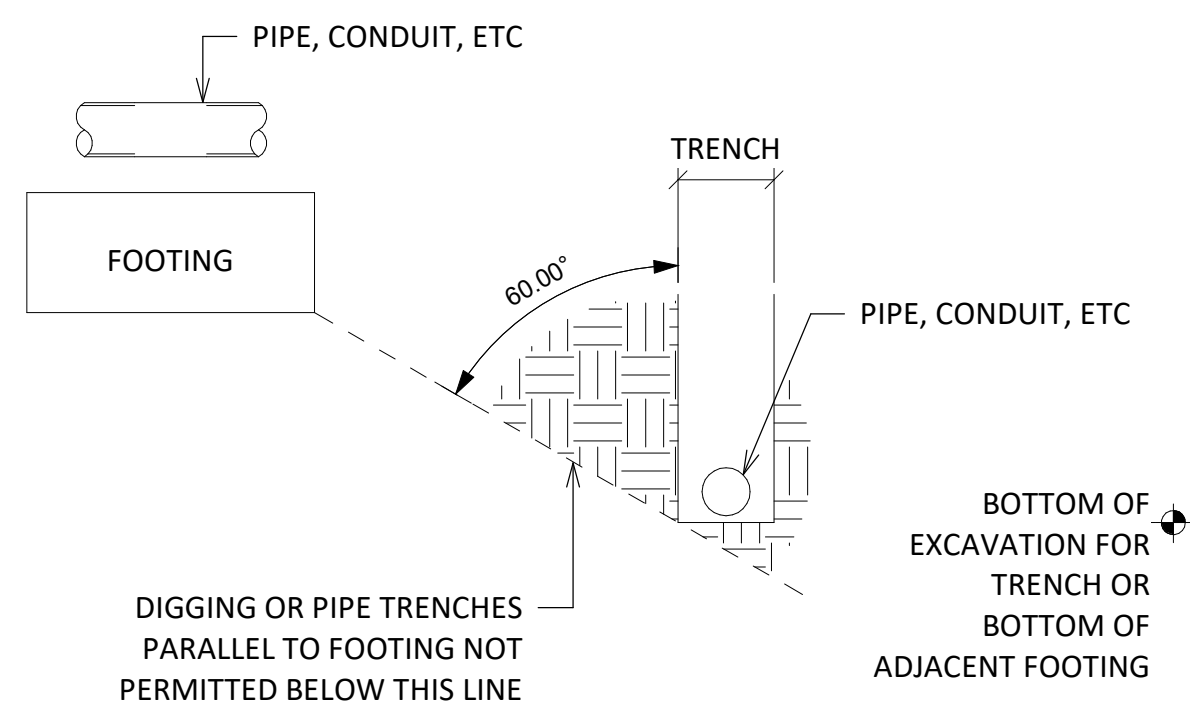
NOTES

1. CJ INDICATES CONTROL JOINT
2. PROVIDE 1/2" PRE-FORMED EXPANSIVE JOINT FILLER AROUND COLUMN.
3. INFILL HATCHED AREA A MINIMUM OF 28 DAYS AFTER CASTING SLAB.
4. COAT STEEL ELEMENTS WITH BITUMINOUS MASTIC BELOW TOP OF SLAB.

(2) #4 X 4'-0" LONG WHERE SLAB CJ IS DISCONTINUOUS. PLACE BARS 1" BELOW TOP OF SLAB



11 SLAB-ON-GRADE CONTROL JOINT - STEEL HSS COLUMN
3/4" = 1'-0"



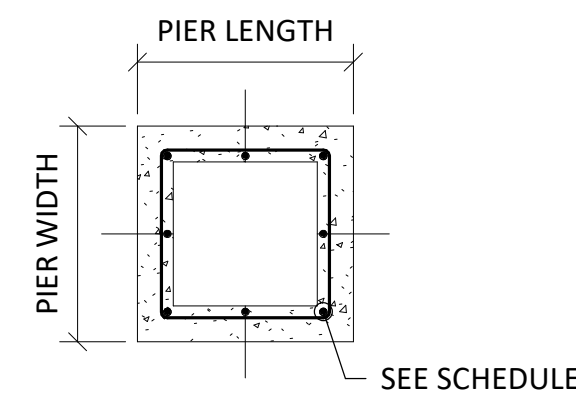
NOTE

LOWER FOOTING AS REQUIRED TO PROVIDE MAX 30° SLOPE FROM BOTTOM EDGE OF FOOTING TO BOTTOM OF TRENCH EXCAVATION OR LOWER FOOTING SO THAT TOP OF FOOTING IS BELOW PIPE, CONDUIT, ETC

12 FOUNDATION INFLUENCE DETAIL
3/4" = 1'-0" TYPICAL DETAIL

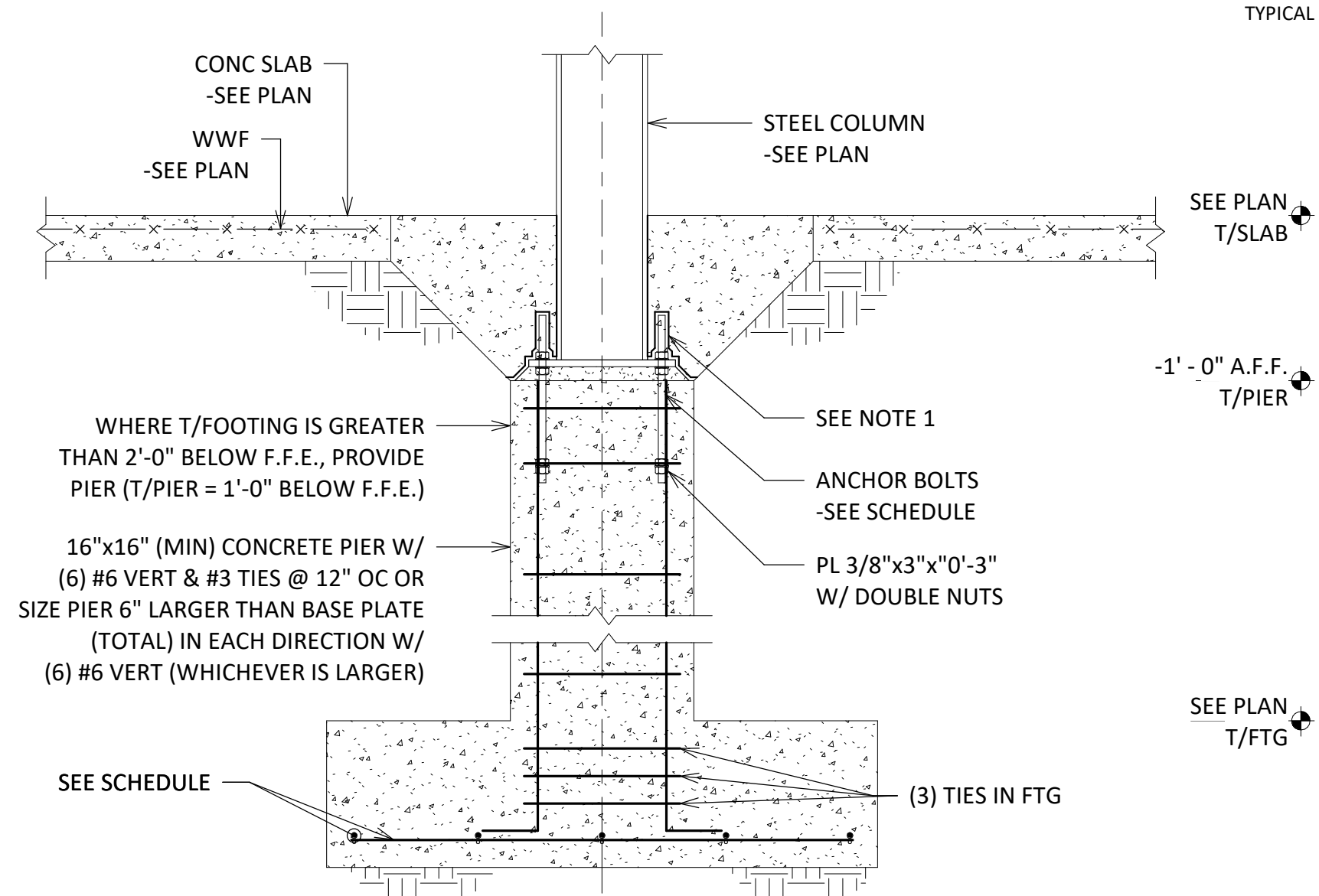
9 SLAB ON GRADE FLOOR DRAIN DETAIL
3/4" = 1'-0" TYPICAL DETAIL

5 STEEL COLUMN ON SPREAD FOOTING DETAIL
3/4" = 1'-0"



PIER LAYOUT

PIER SCHEDULE		
PIER ID	PIER SIZE (INCHES)	VERT REINF.



NOTES

1. SEPARATE SLAB FROM FOOTING W/ 2 LAYERS OF PVC VAPOR BARRIER OR 1/2" EXPANSIVE JOINT FILLER.
2. COAT BELOW GRADE PORTION OF COLUMN & BASE PLATE W/ BITUMINOUS MASTIC.

6 STEEL COLUMN ON CONCRETE PIER DETAIL
3/4" = 1'-0" TYPICAL DETAIL

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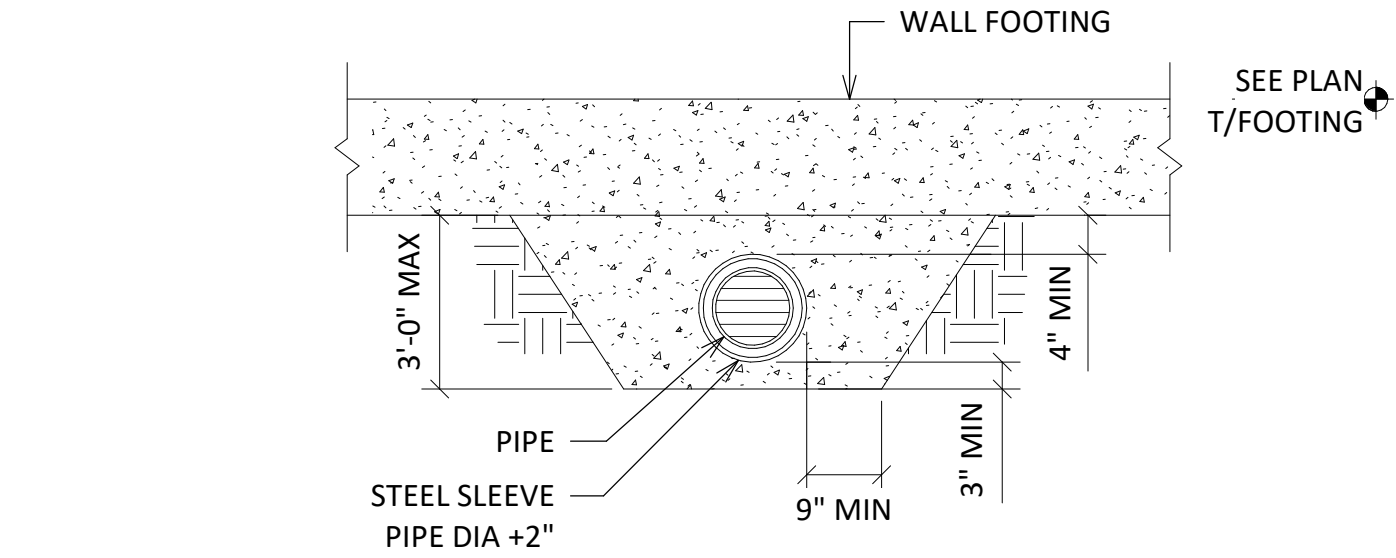
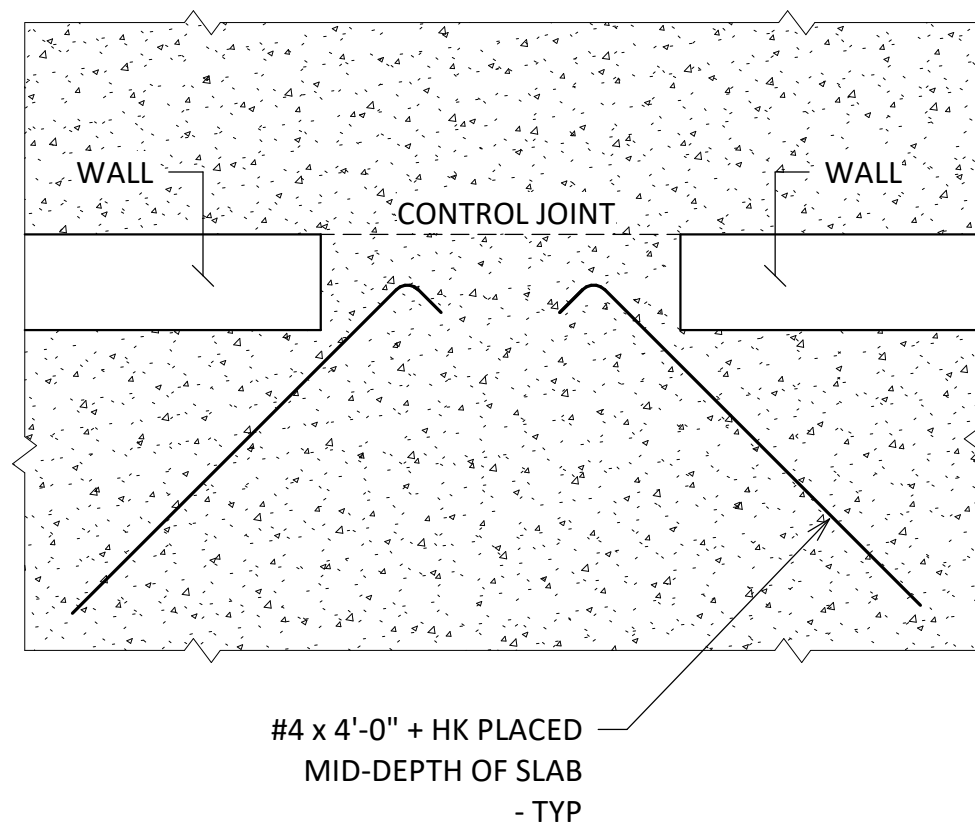
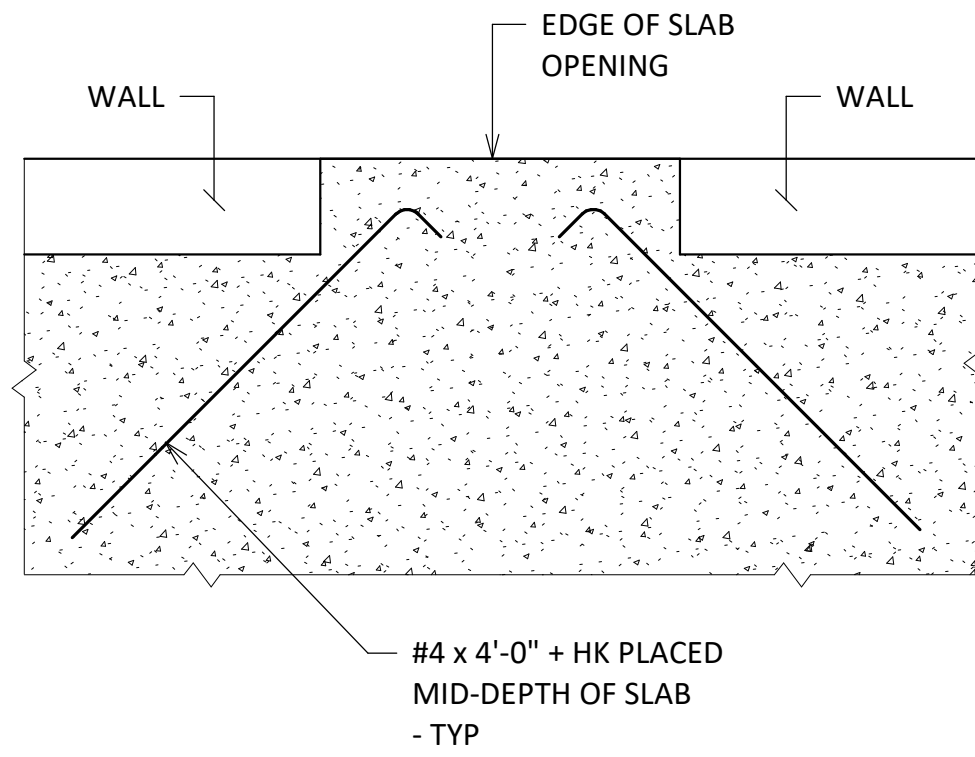
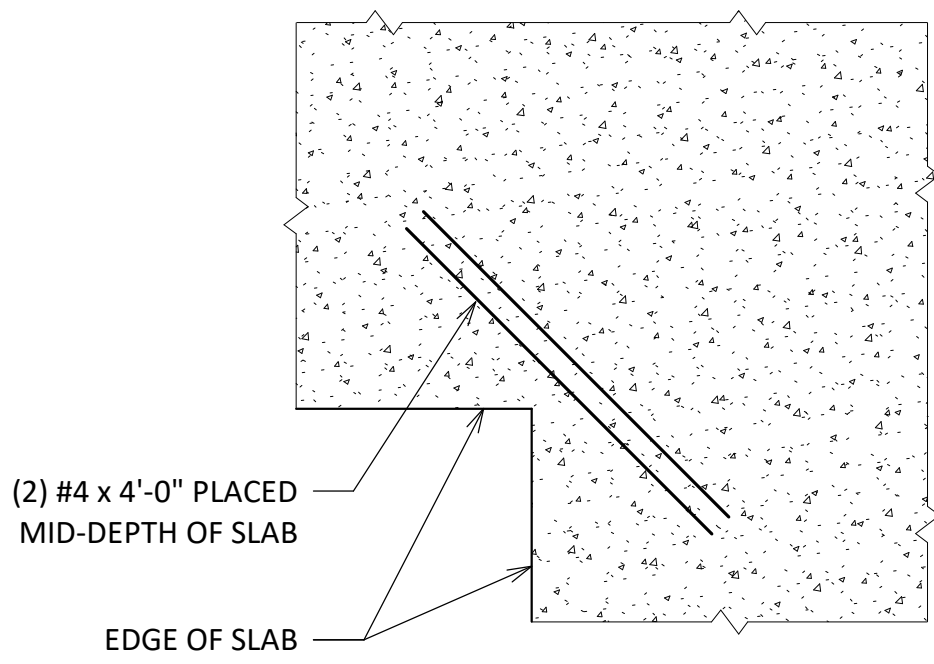
SHEET TITLE:

TYPICAL FOUNDATION DETAILS

PROJECT NO: 25021

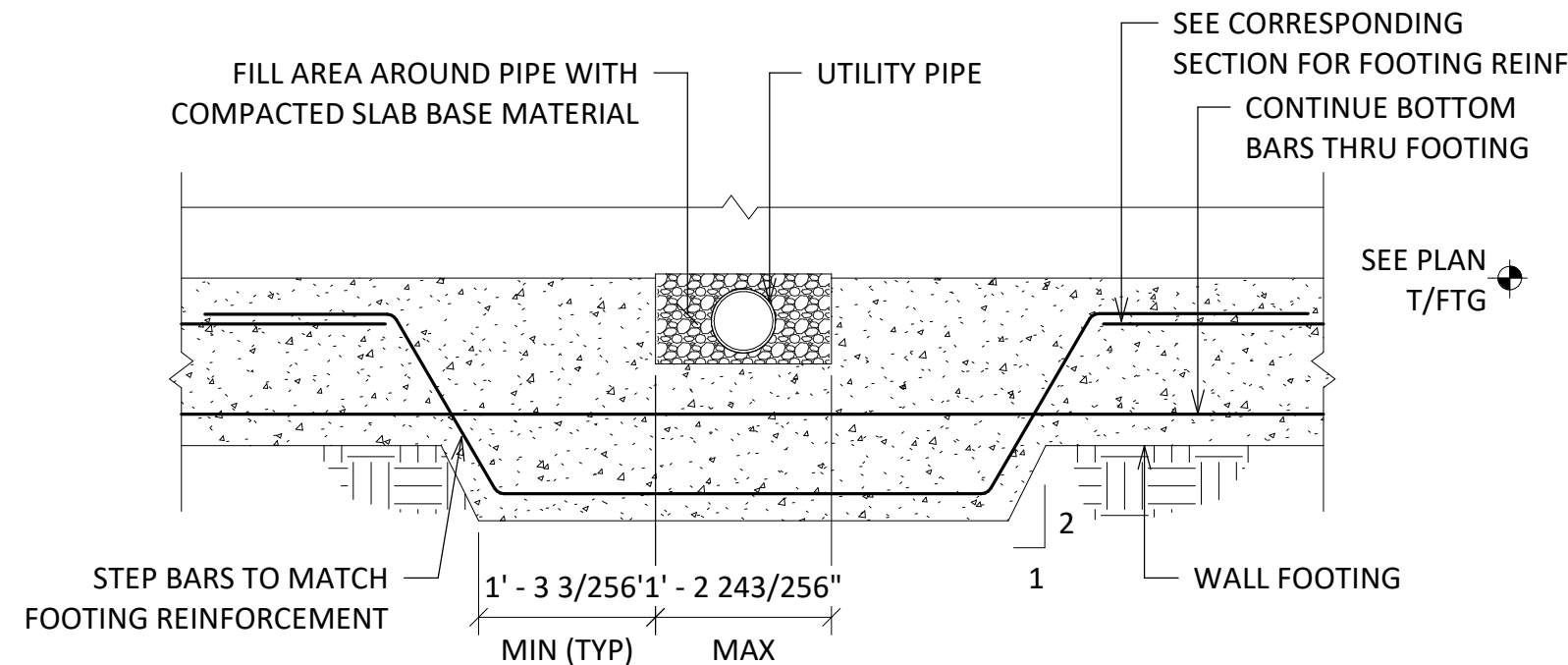
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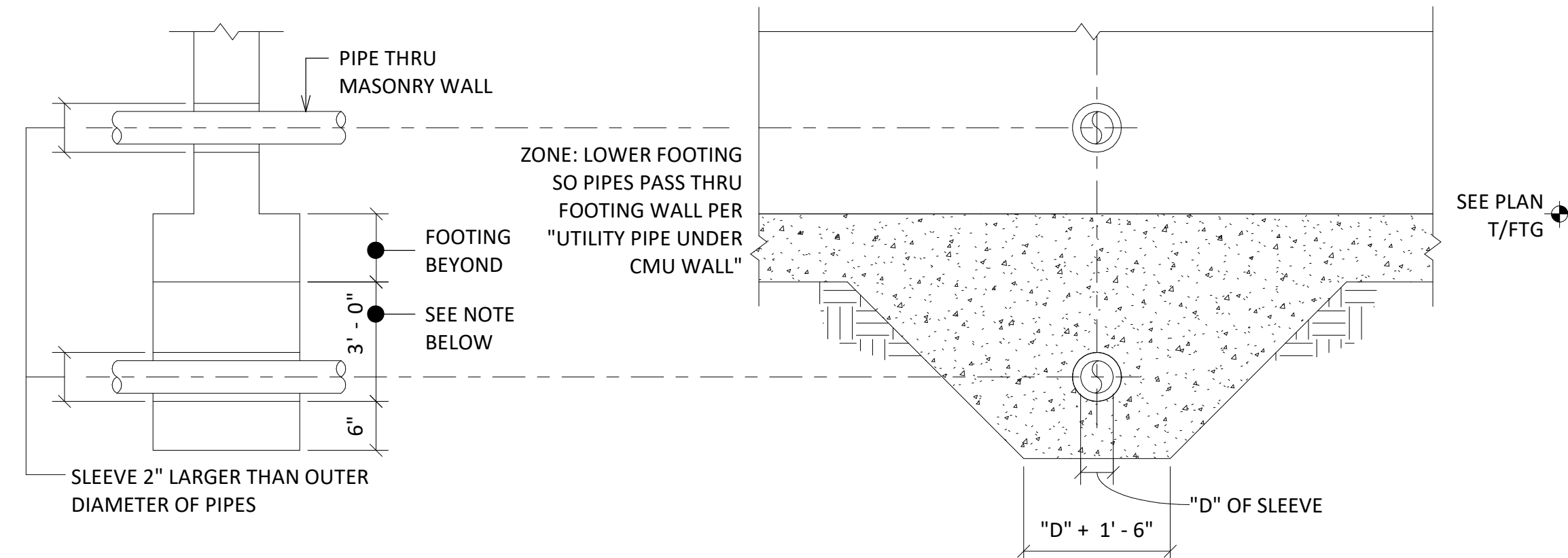


NOTE
DETAIL TO BE APPLIED WHEN TOP OF PIPE IS 3'-0" OR CLOSER TO UNDERSIDE OF FOOTING. WHEN CLEARANCE IS GREATER THAN 3'-0", BACKFILL WITH #57 STONE.

PIPE UNDER WALL FOOTING



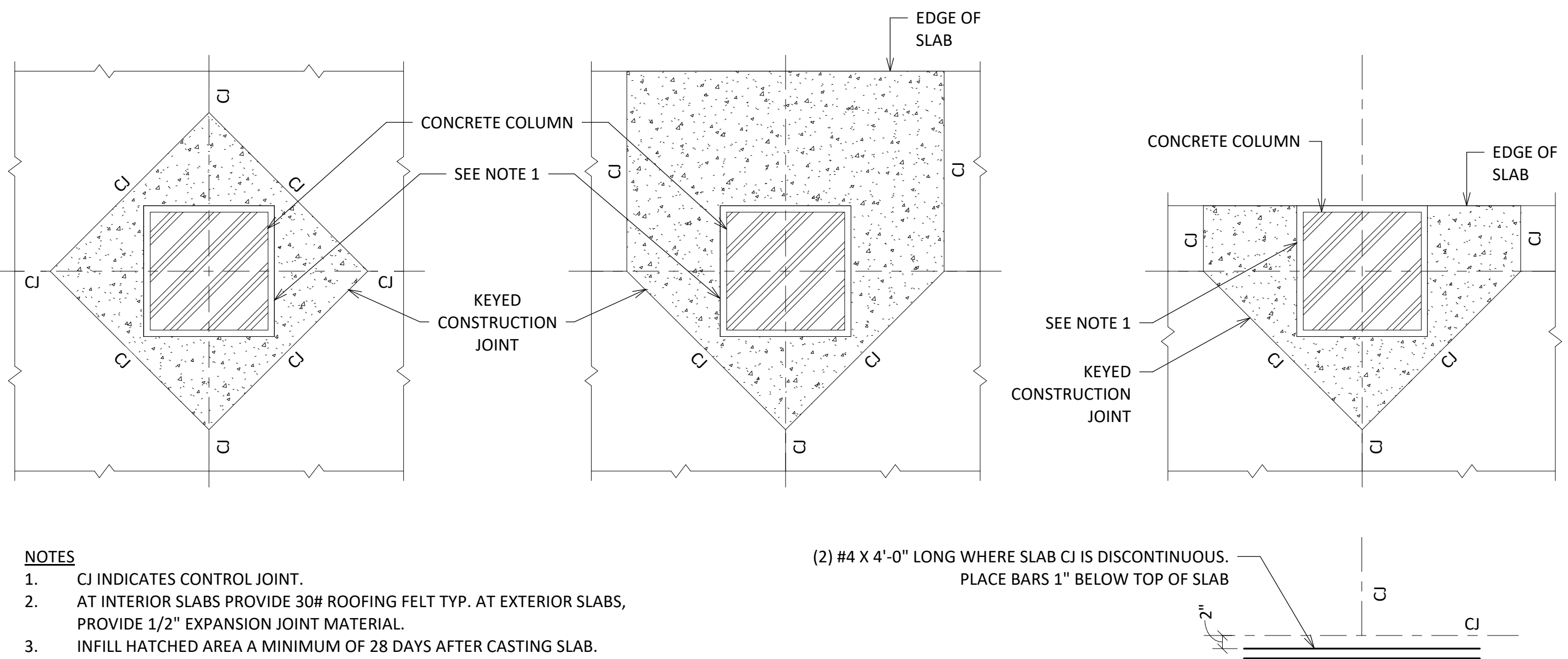
UTILITY PIPE UNDER BEARING WALL



NOTE
FOR PIPES 3'-0" OR LESS BELOW FOOTING, PROVIDE SLEEVE & CONCRETE AS SHOWN; FOR PIPES MORE THAN 3'-0" BELOW BOTTOM OF FOOTING COMPACT BACKFILL OVER PIPE TO 90% PROCTOR DENSITY (UNO)

EXCAVATION TRANSVERSE TO WALL FOOTING

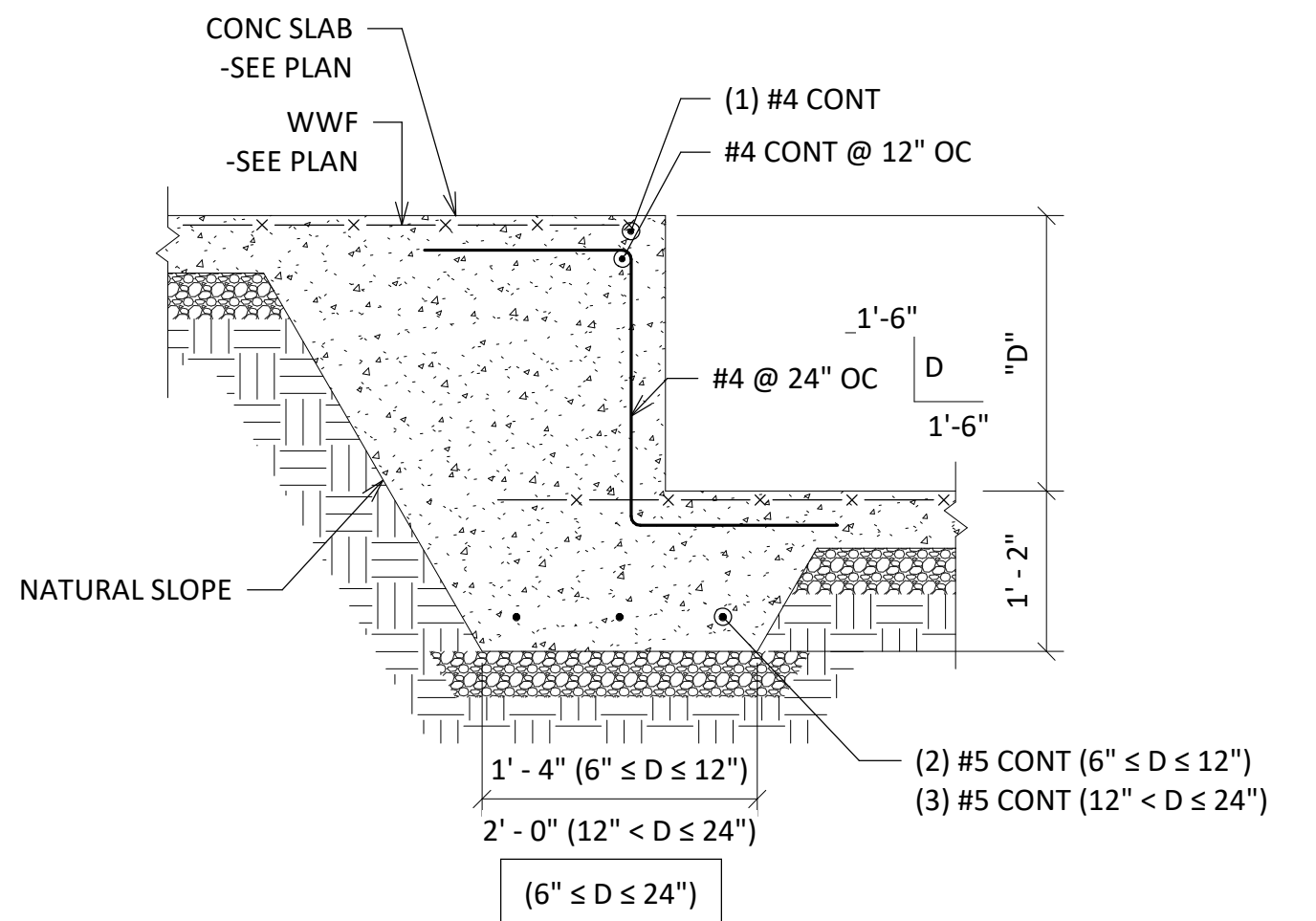
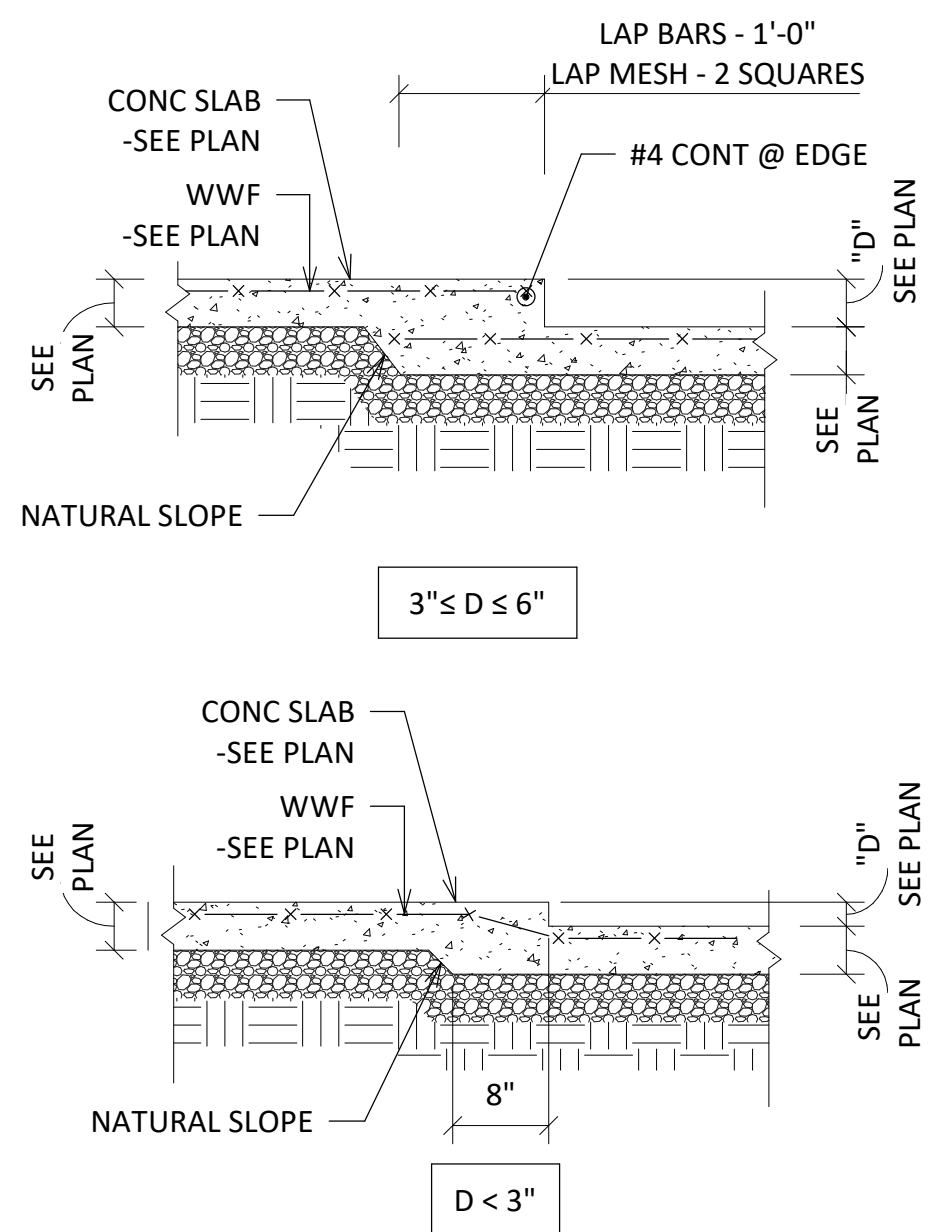
5 FOUNDATION AT UTILITY PIPES DETAIL
3/4" = 1'-0"



NOTES
1. CJ INDICATES CONTROL JOINT.
2. AT INTERIOR SLABS PROVIDE 30# ROOFING FELT TYP. AT EXTERIOR SLABS, PROVIDE 1/2" EXPANSION JOINT MATERIAL.
3. INFILL HATCHED AREA A MINIMUM OF 28 DAYS AFTER CASTING SLAB.

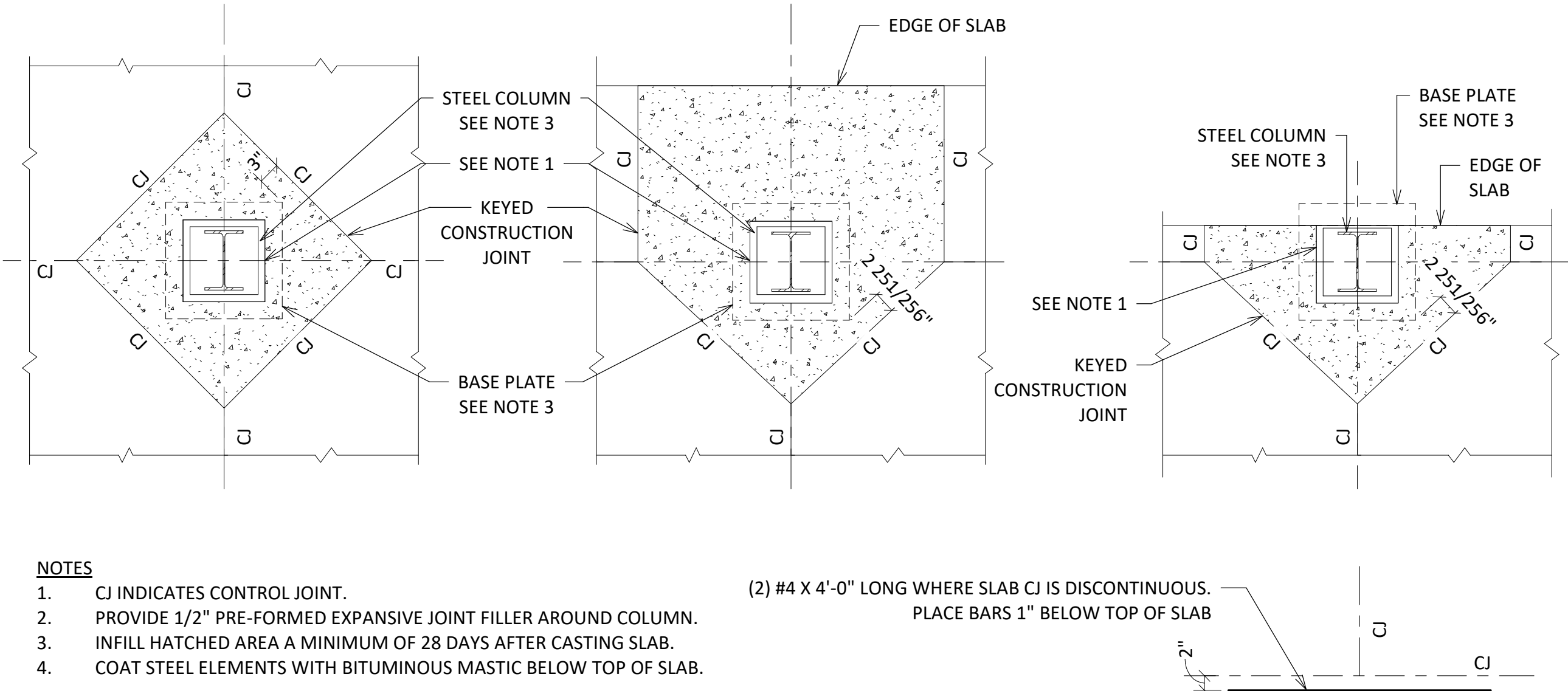
(2) #4 X 4'-0" LONG WHERE SLAB CJ IS DISCONTINUOUS. PLACE BARS 1" BELOW TOP OF SLAB

10 SLAB ON GRADE REINF AT INTERIOR OPENINGS
3/4" = 1'-0"



NOTES
1. WWF TO BE POSITIONED WITHIN SLAB DEPTH WITH PREFABRICATED CHAIRS.
2. COORDINATE DEPTHS AND LOCATIONS OF ALL FLOOR DEPRESSIONS WITH ARCHITECTURAL DRAWINGS.
3. PROVIDE (1) #4 X 4'-0" TOP AT INTERIOR CORNERS OF ALL DEPRESSIONS.
4. SLAB DEPRESSIONS ARE TYPICALLY SHOWN ON PLAN THUS:

11 DEPRESSED SLAB DETAILS
3/4" = 1'-0"



NOTES
1. CJ INDICATES CONTROL JOINT.
2. PROVIDE 1/2" PRE-FORMED EXPANSIVE JOINT FILLER AROUND COLUMN.
3. INFILL HATCHED AREA A MINIMUM OF 28 DAYS AFTER CASTING SLAB.
4. COAT STEEL ELEMENTS WITH BITUMINOUS MASTIC BELOW TOP OF SLAB.

(2) #4 X 4'-0" LONG WHERE SLAB CJ IS DISCONTINUOUS. PLACE BARS 1" BELOW TOP OF SLAB

12 SLAB-ON-GRADE CONTROL JOINT - STEEL W COLUMN
3/4" = 1'-0"

6 SLAB-ON-GRADE CONTROL JOINT - CONCRETE COLUMN
3/4" = 1'-0"

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JEFFERSONVILLE
GEORGIA 31044

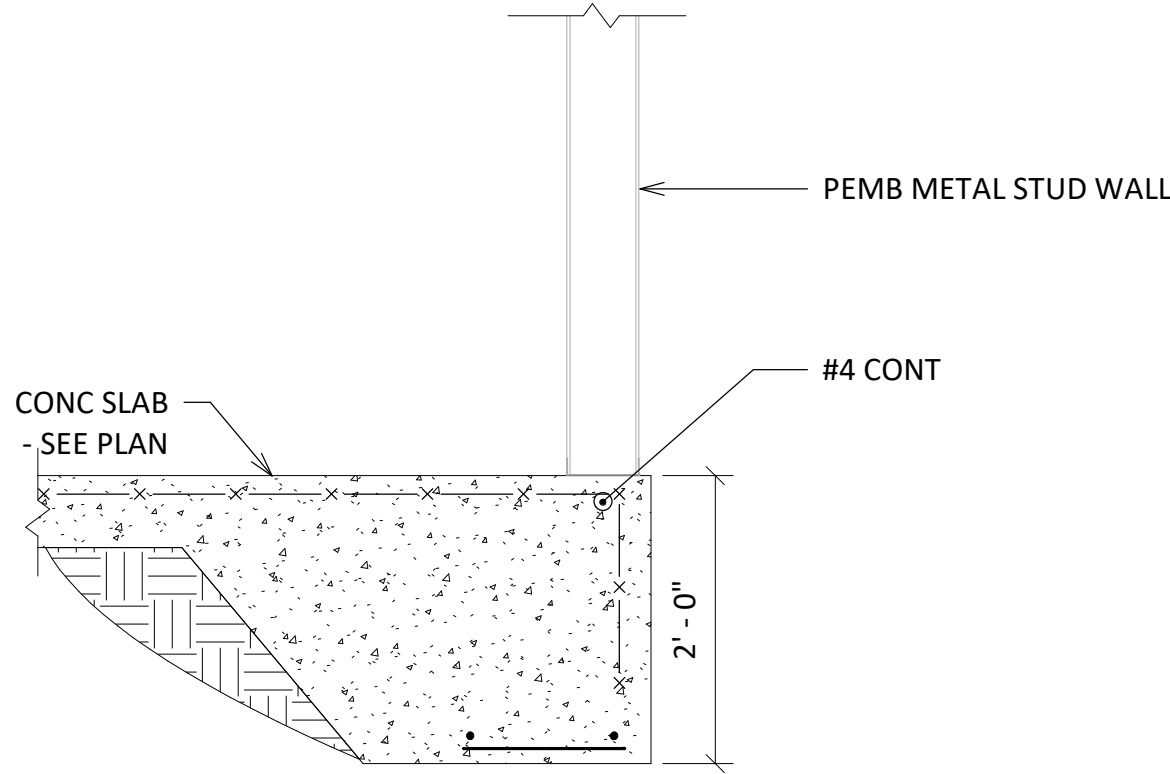
SHEET TITLE:

TYPICAL FOUNDATION DETAILS

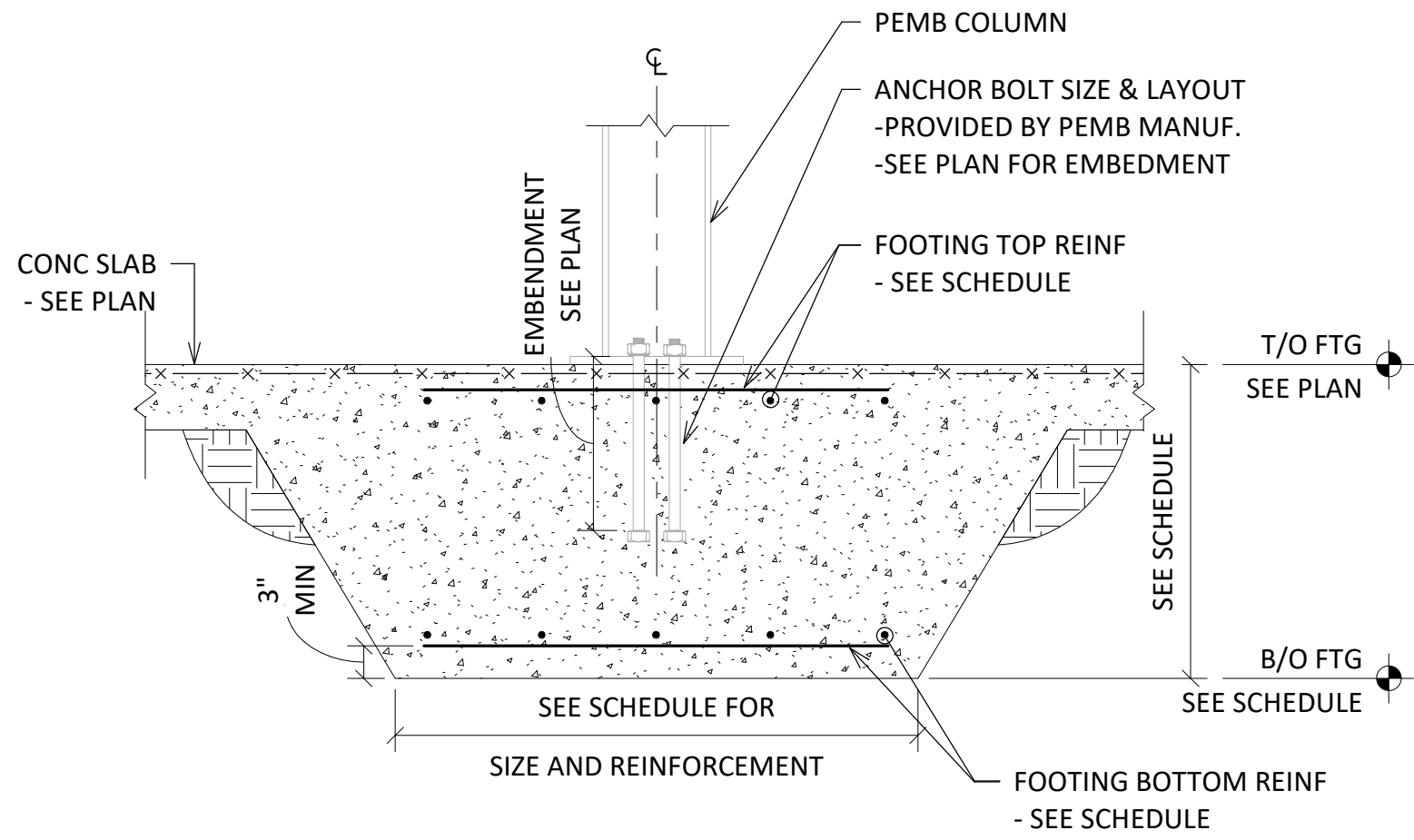
PROJECT NO: 25021

S202

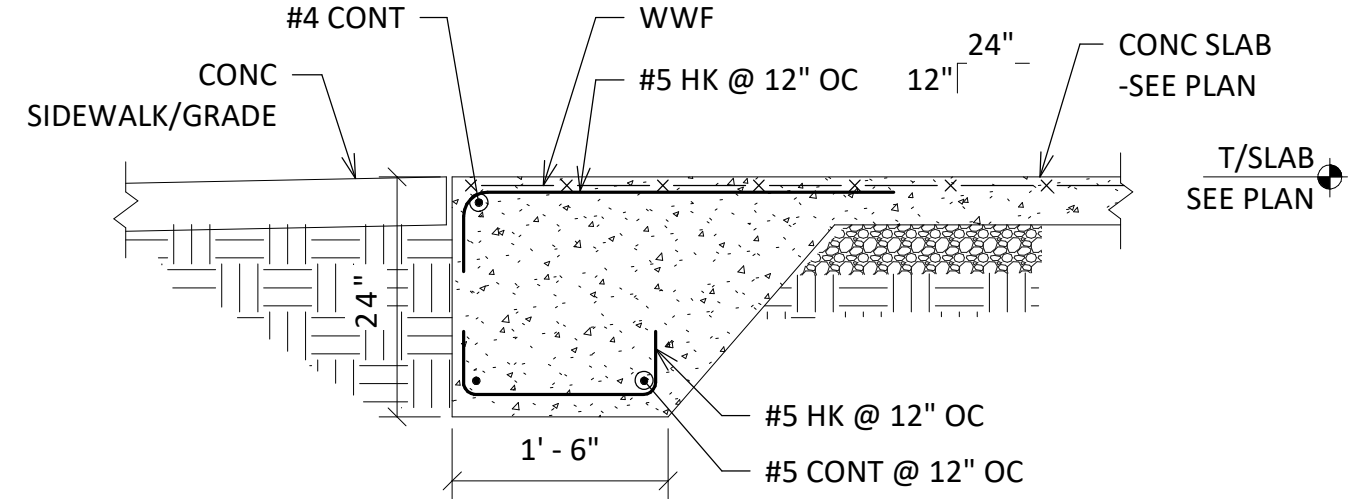
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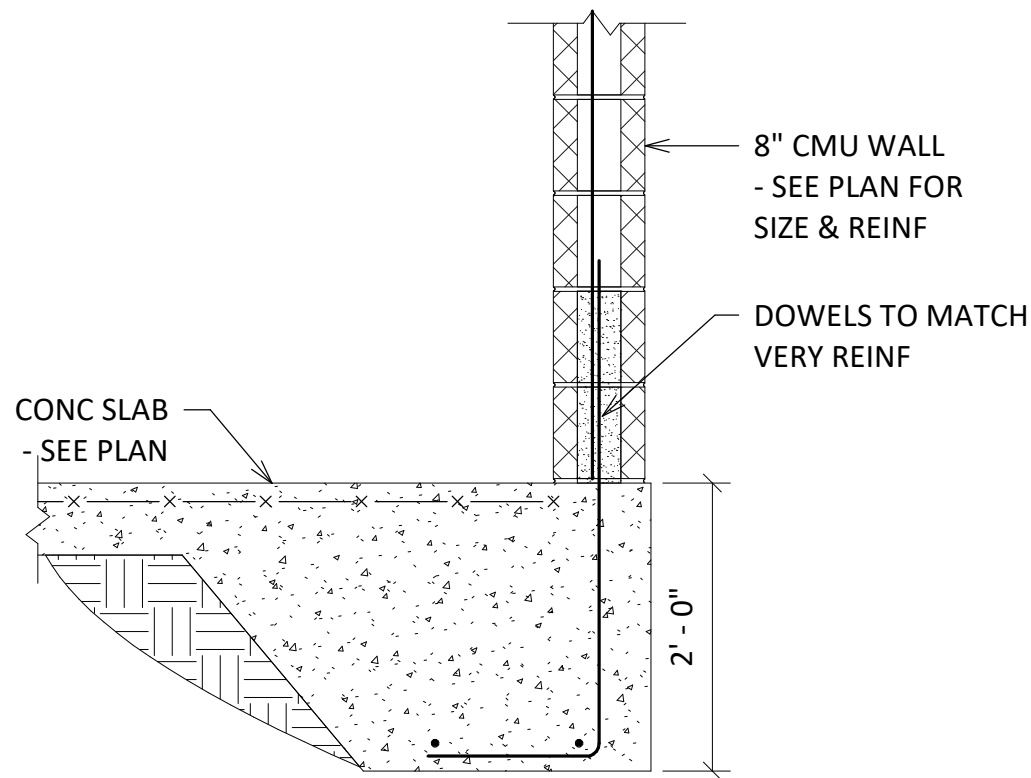
7 SECTION THRU MTL STD WALL FOOTING
3/4" = 1'-0"



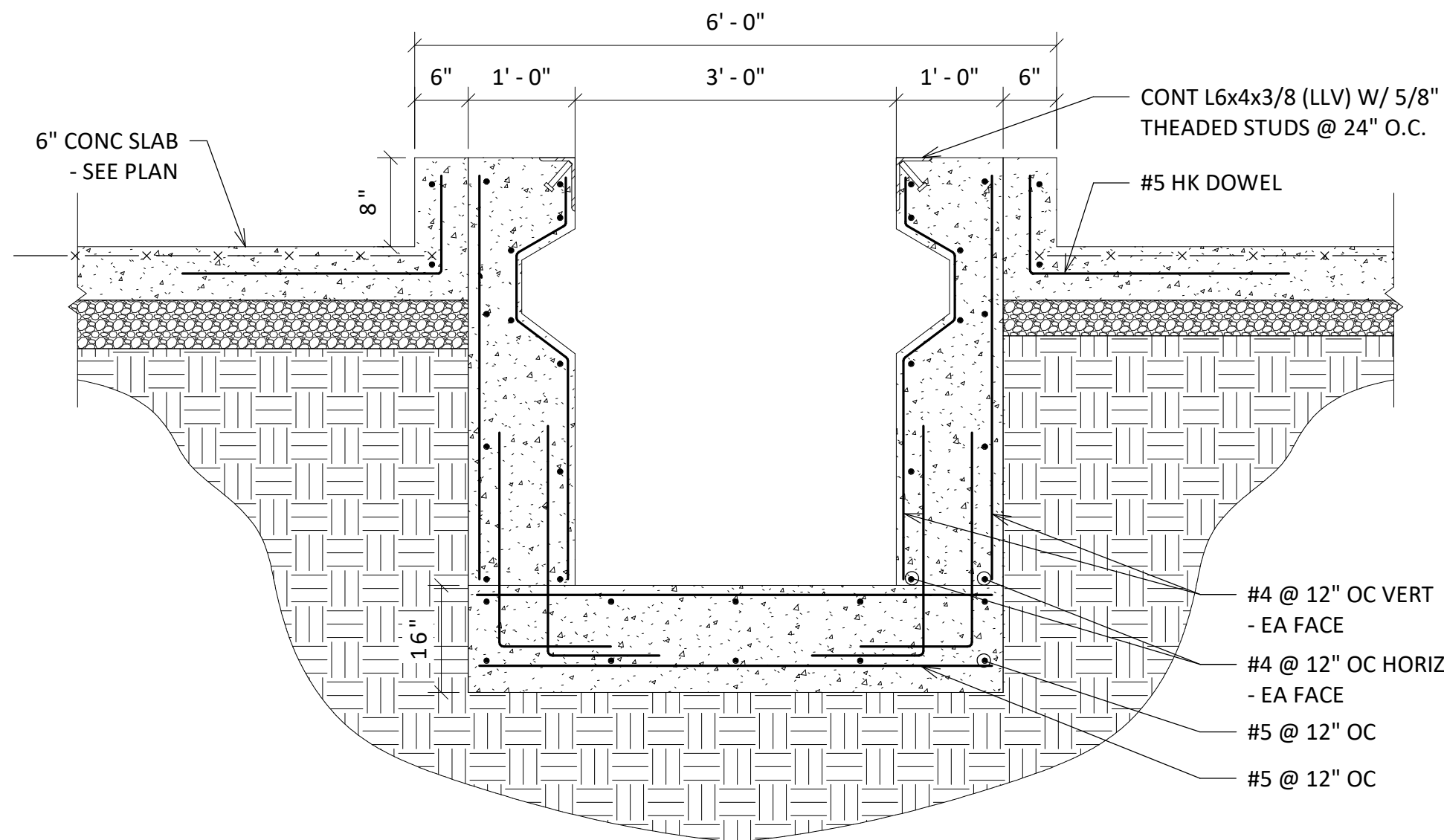
4 SECTION THRU INT COLUMN FOOTING
3/4" = 1'-0"



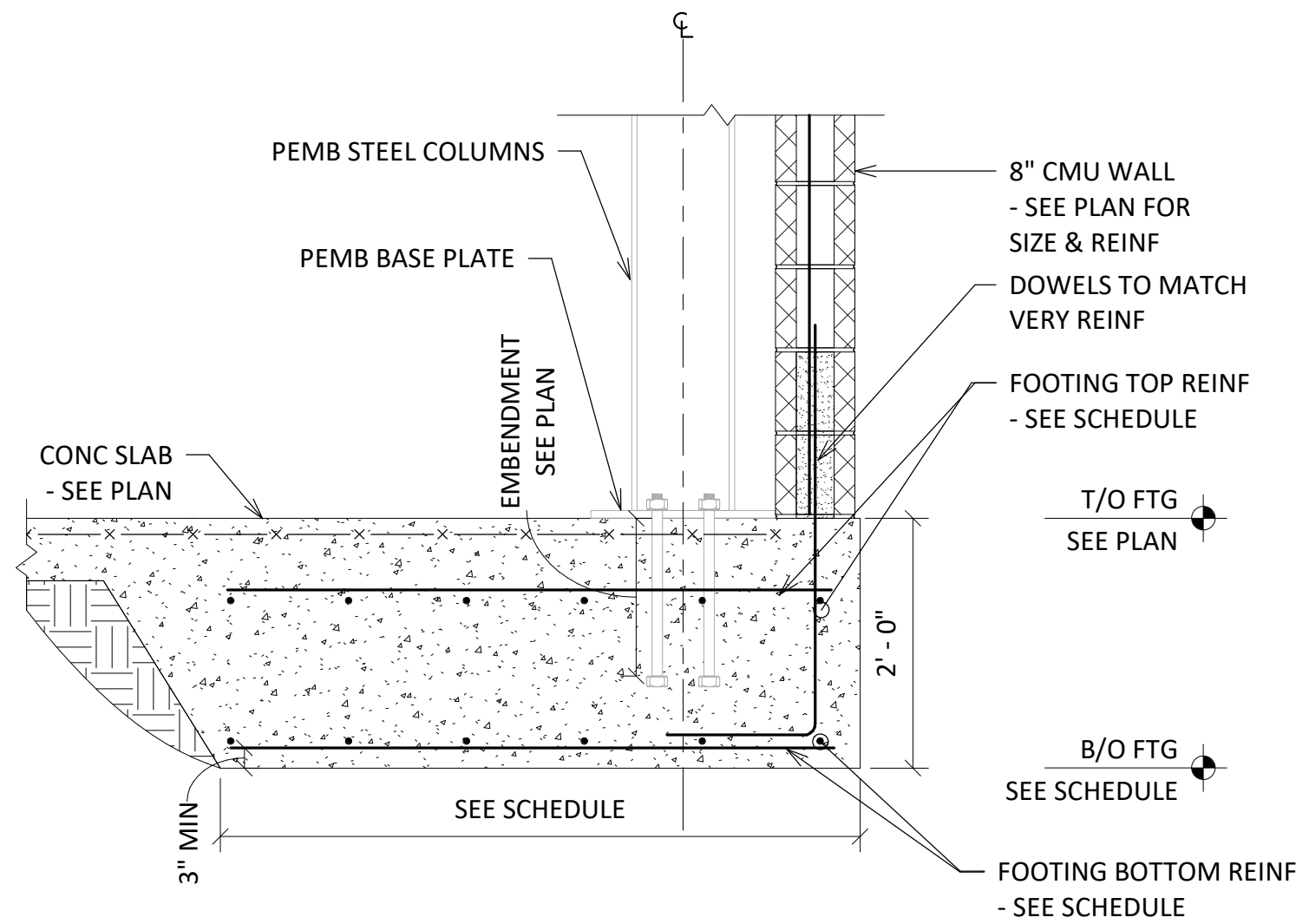
1 SECTION AT OPENING
3/4" = 1'-0"



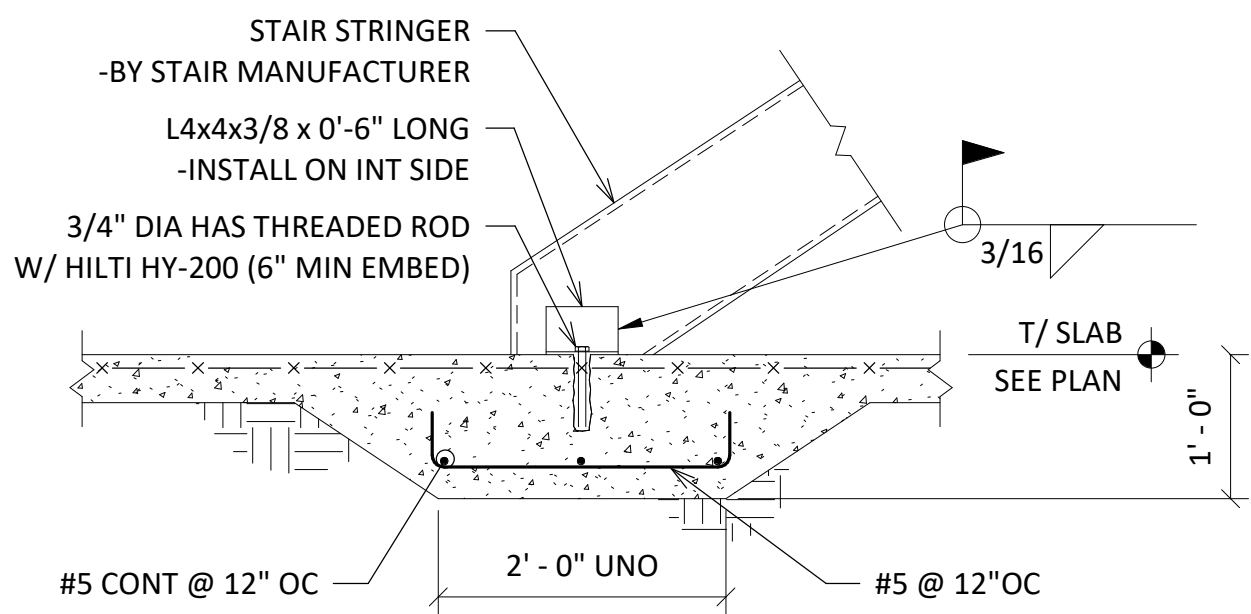
8 SECTION THRU CMU WALL FOOTING
3/4" = 1'-0"



5 SECTION THRU MAINTENANCE PIT
3/4" = 1'-0"

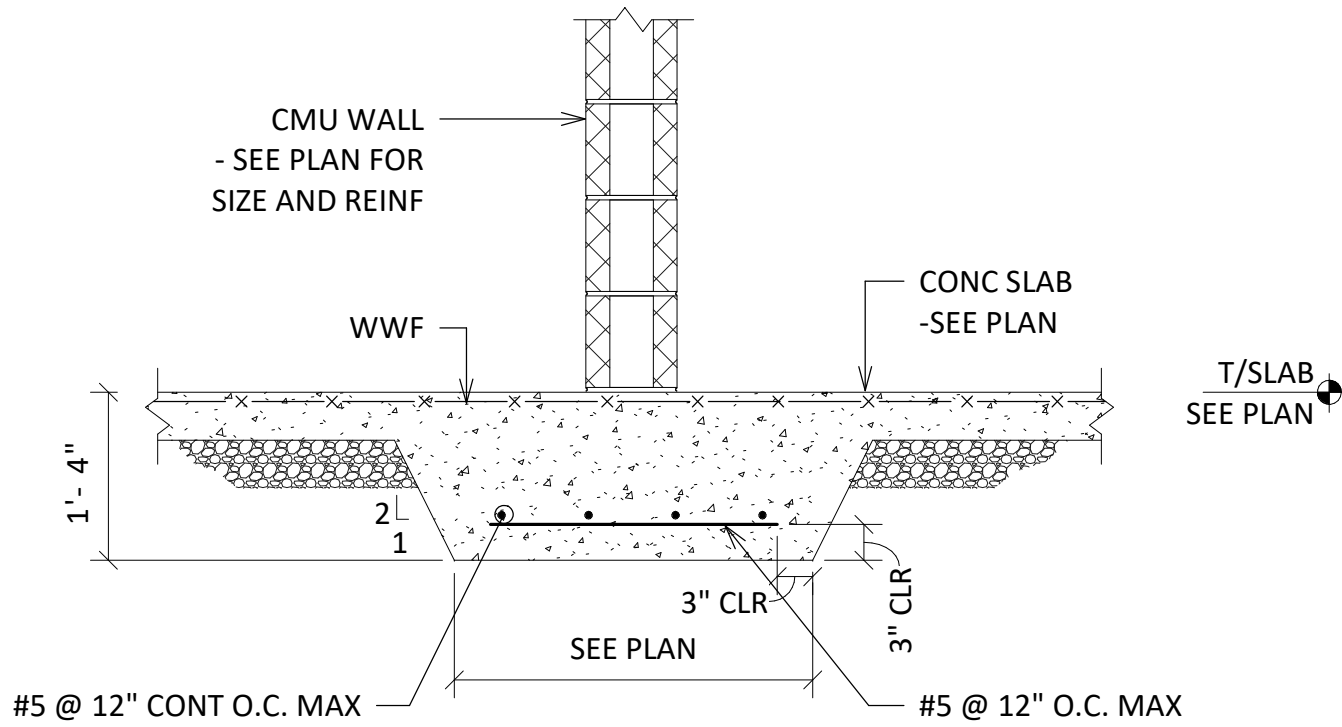


2 SECTION THRU FOOTING W/ CMU WALL
3/4" = 1'-0"

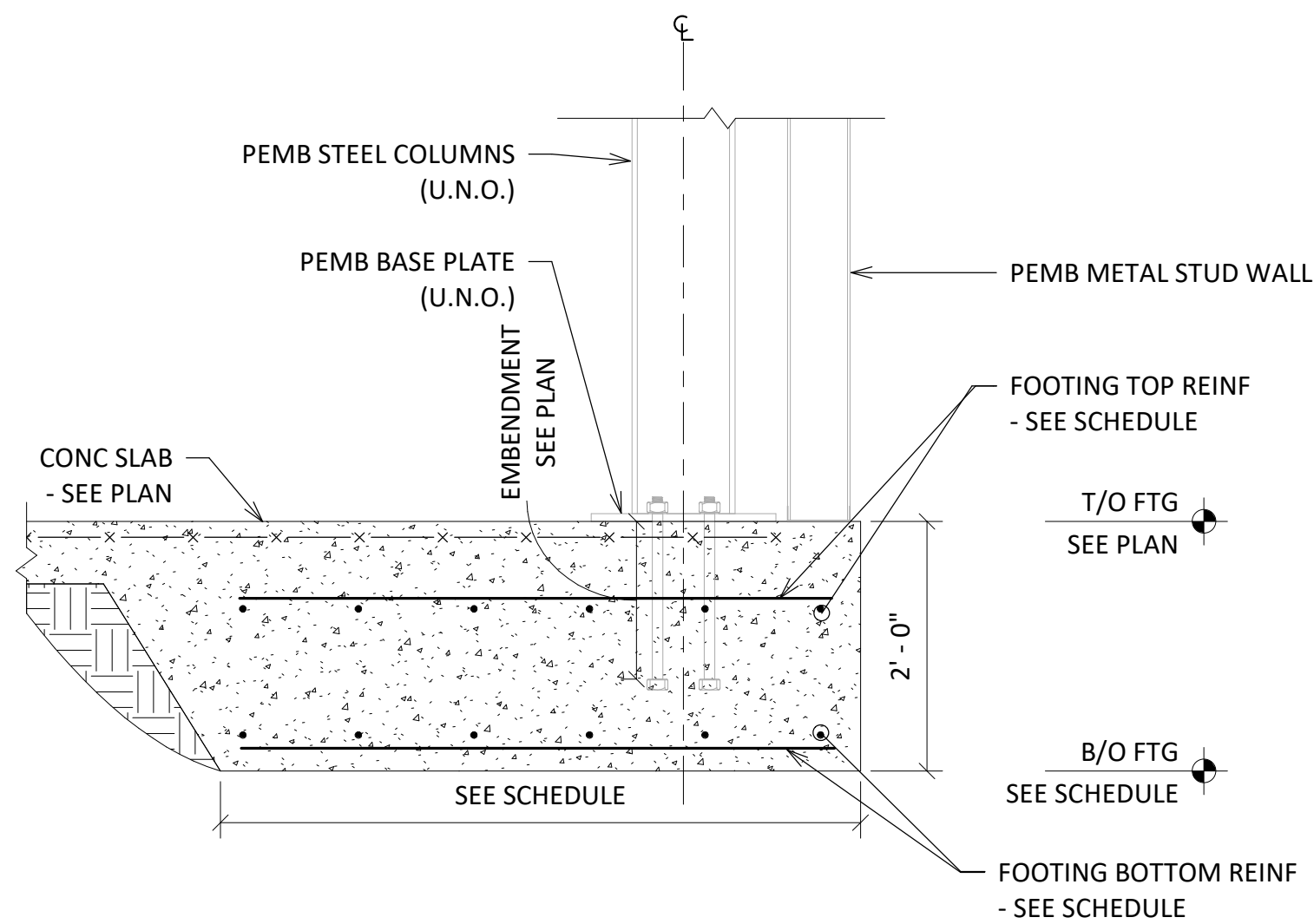


- NOTES
- CONTRACTOR IS RESPONSIBLE FOR THE COORDINATION OF STAIR STRINGER LOCATIONS WITH ARCH'L DRAWINGS.
 - CLIP ANGLE TO BE HIDDEN BENEATH FIRST TREAD, UNO.

9 THICKENED SLAB AT STEEL STAIR STRINGER
3/4" = 1'-0"



6 SECTION AT INTERIOR CMU BEARING WALL
3/4" = 1'-0"



3 SECTION THRU FOOTING W/ METAL STUD WAL
3/4" = 1'-0"

ARCHITECT OF RECORD:

THOMAS E. MORGAN, JR.
ARCHITECT

423 FISCHER TRAIL
ELLIJAY, GEORGIA 30540

SEAL:



6/29/2026



3450 Acworth Due West Road
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Kennesaw, Georgia 30144
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RELEASE FOR CONSTRUCTION

ISSUES / REVISIONS:

DATE	DESCRIPTION
06/29/2026	PERMIT SET

DRAWN BY: CB

CHKD BY: MM

PROJECT DESCRIPTION:

PROJECT TITLE:

TRAVEL CENTER
5524 GA HIGHWAY 96 W
JEFFERSONVILLE
GEORGIA 31044

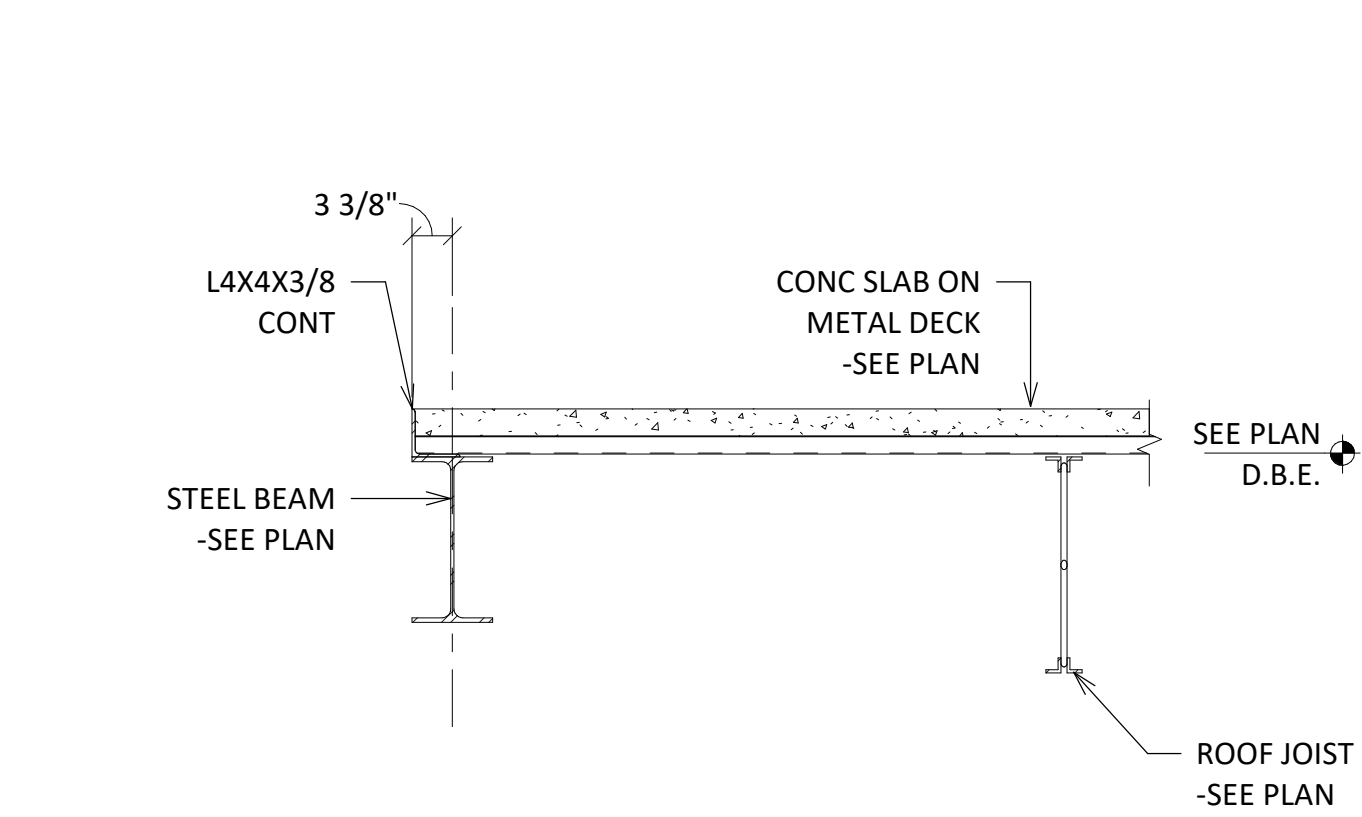
SHEET TITLE:

INTERIOR
FOUNDATION
DETAILS

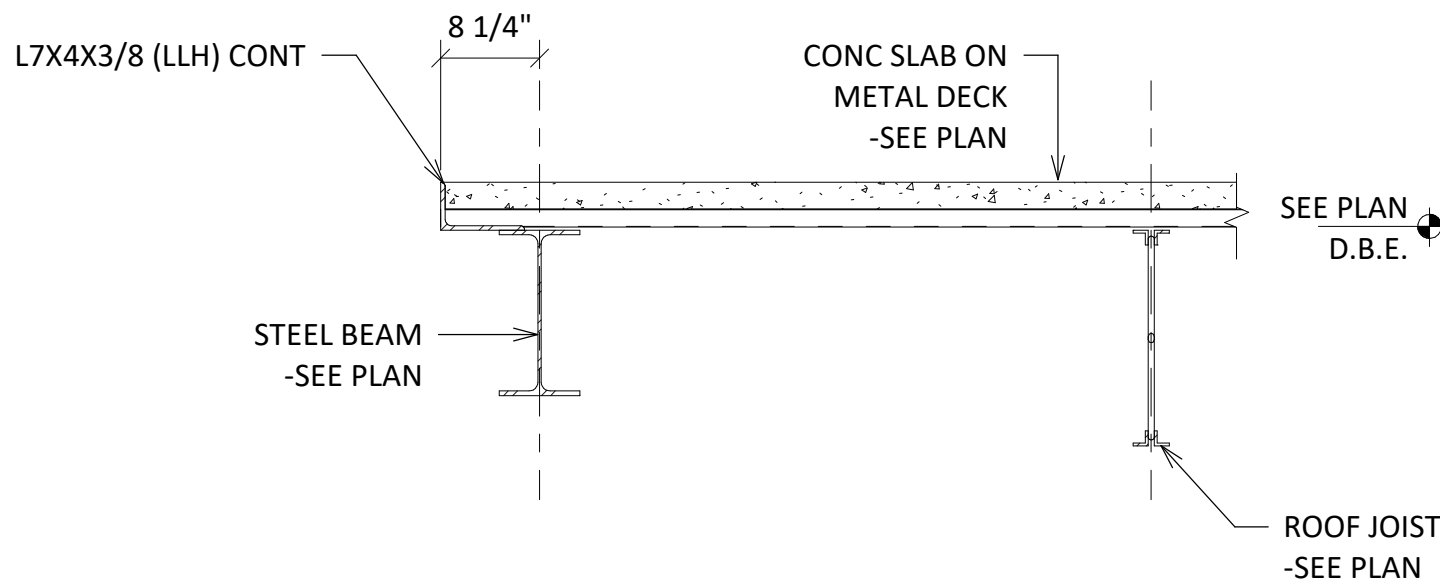
PROJECT NO: 25021

S203

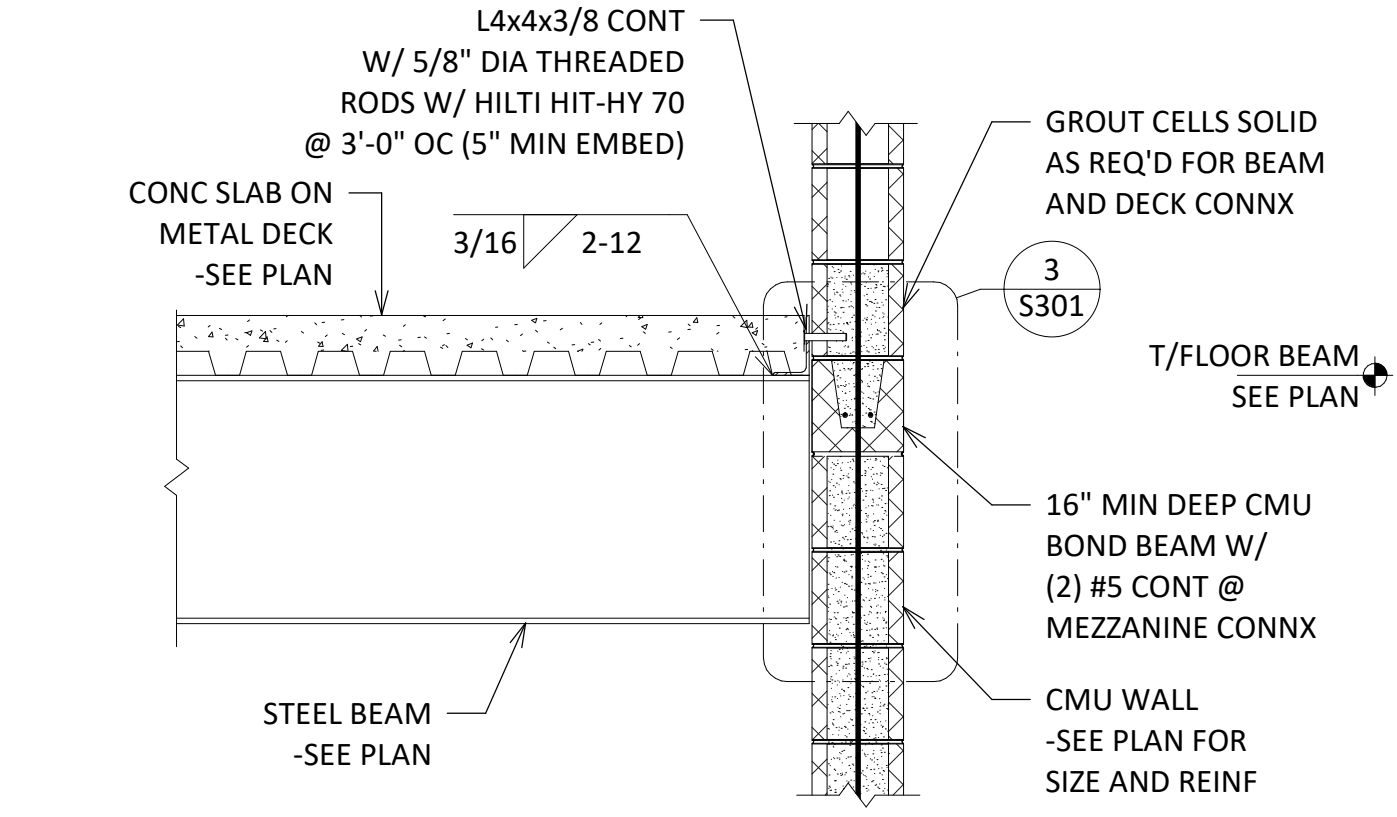
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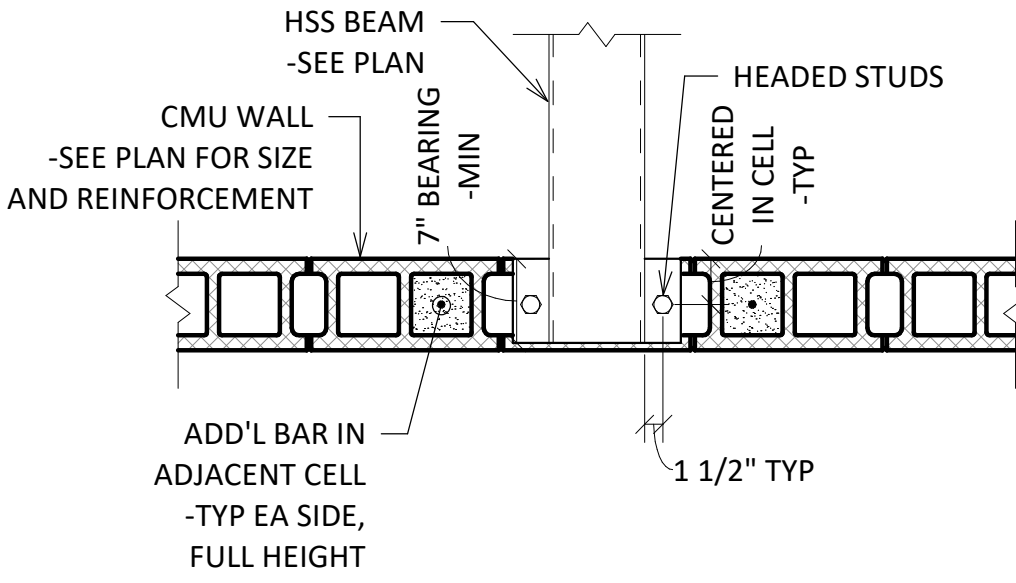
6 KICKER AT W-BEAM
3/4" = 1'-0"



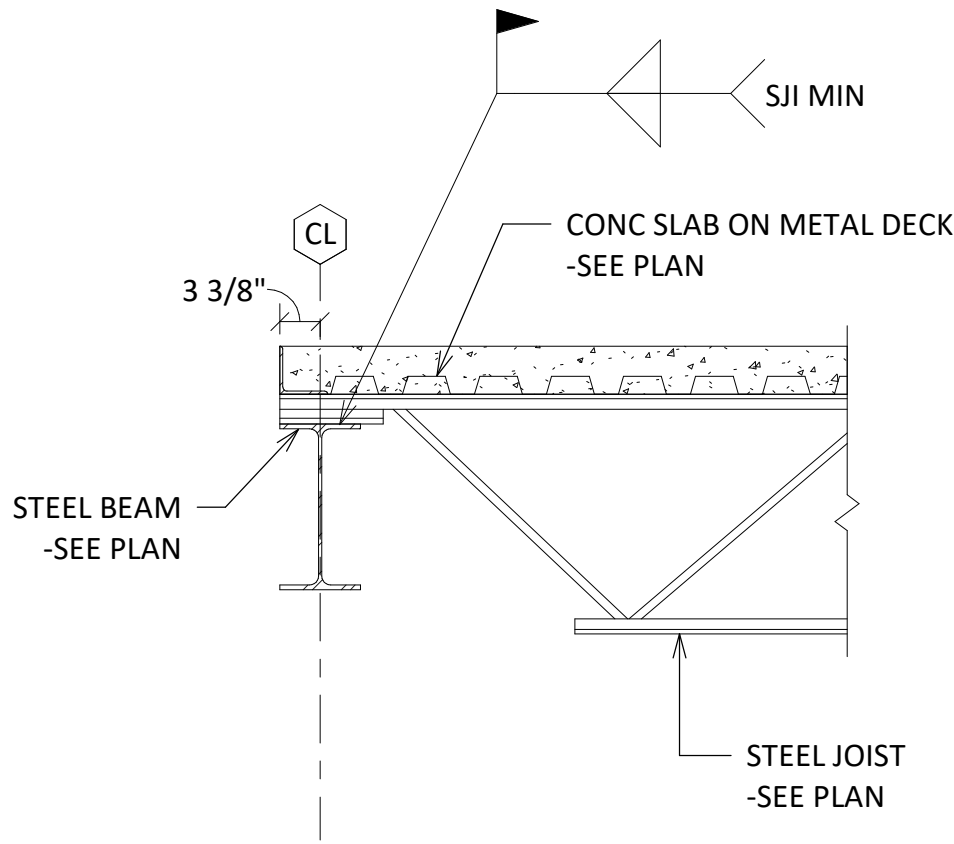
7 KICKER AT W-BEAM
3/4" = 1'-0"



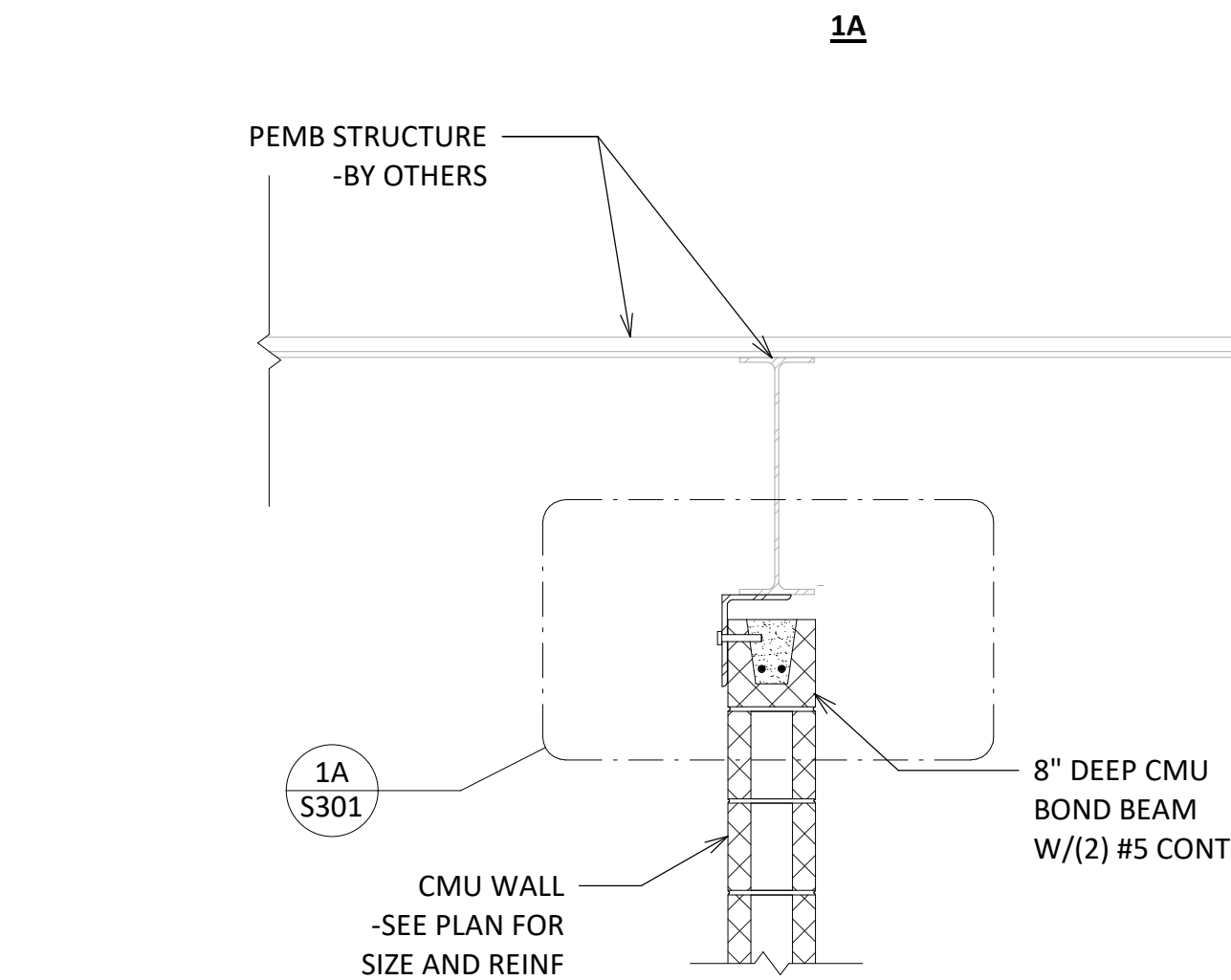
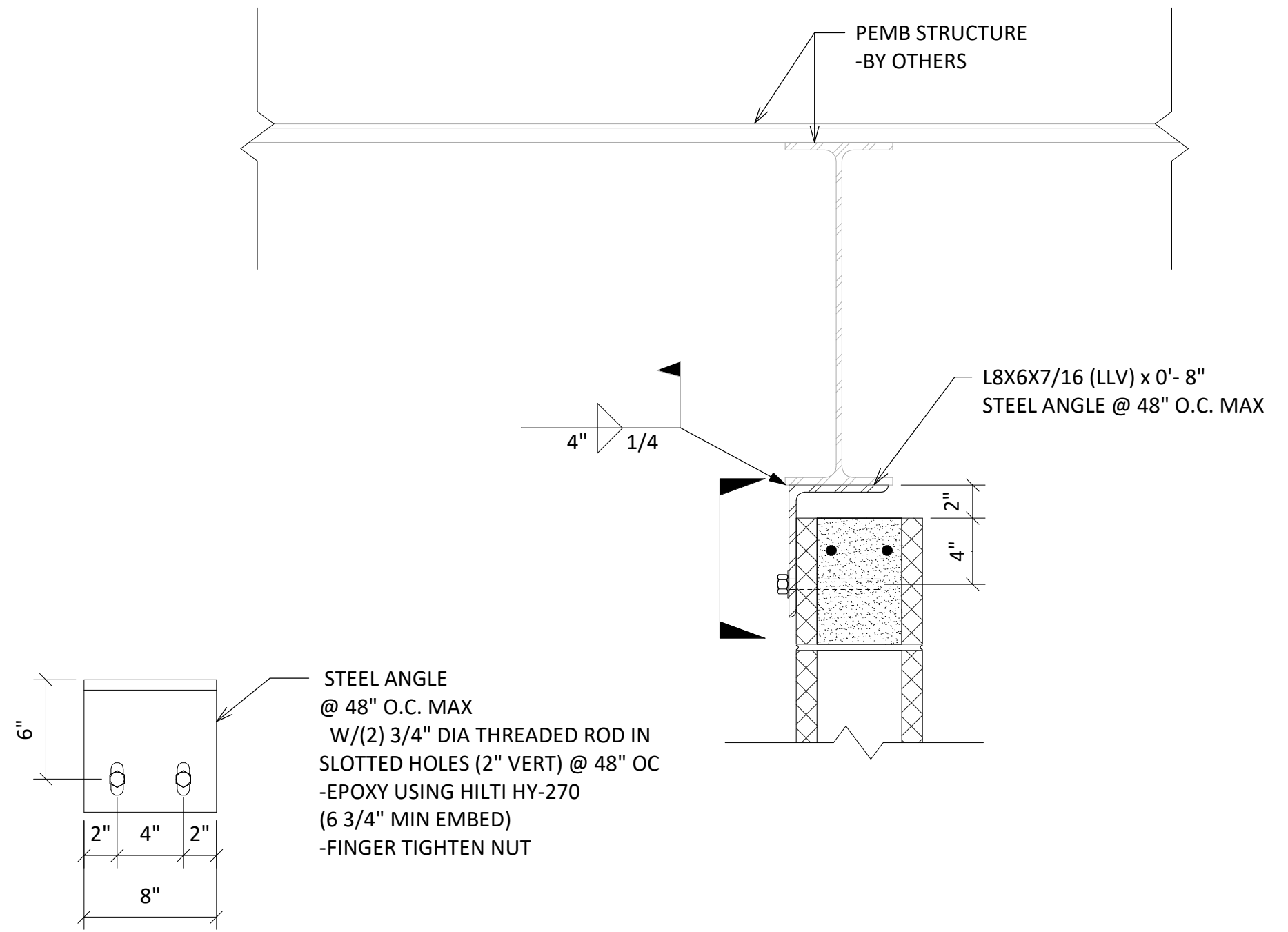
4 PERP CMU WALL SECTION AT COMPOSITE FLOOR
3/4" = 1'-0"



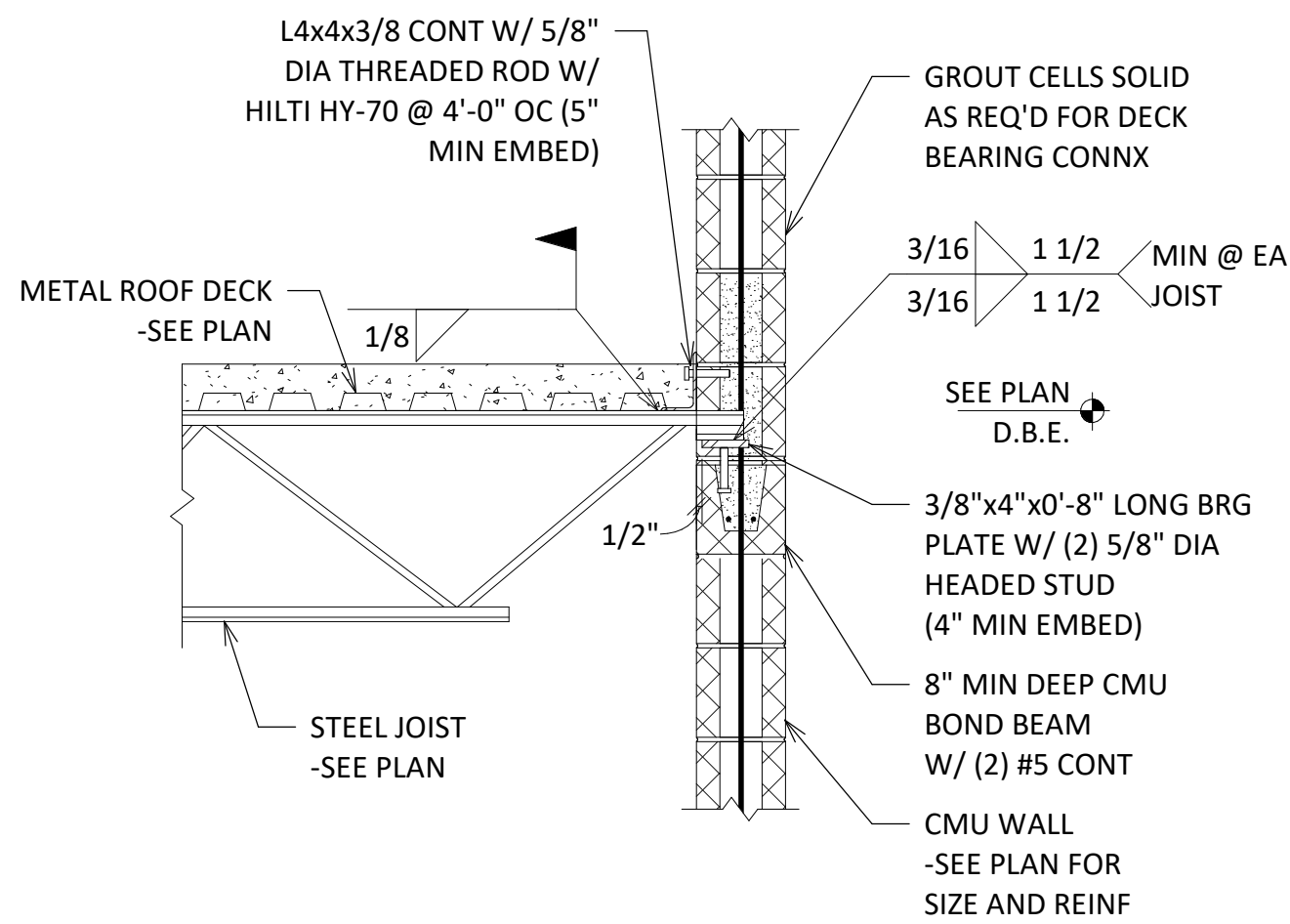
3 STEEL BEAM TO CONC CONNX
3/4" = 1'-0"



5 SECTION AT MEZZANINE FRAMING PERP TO BEAM
3/4" = 1'-0"



1 CMU WALL BRACING SECTION
3/4" = 1'-0"



2 TYPICAL JOIST BEARING AT CMU WALL
3/4" = 1'-0"

ARCHITECT OF RECORD:

THOMAS E. MORGAN, JR.
ARCHITECT

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SEAL:



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RELEASE
FOR CONSTRUCTION

ISSUES / REVISIONS:

DATE	DESCRIPTION	PERMIT SET
06/29/2026	PERMIT SET	

DRAWN BY: CB

CHKD BY: MM

PROJECT DESCRIPTION:

PROJECT TITLE:

TRAVEL CENTER
5524 GA HIGHWAY 96 W
JEFFERSONVILLE
GEORGIA 31044

SHEET TITLE:

FRAMING
SECTIONS
&
DETAILS

PROJECT NO: 25021

S301