GENERAL

1. SEE PROJECT SPECIFICATIONS FOR REQUIREMENTS IN ADDITION TO GENERAL NOTES. COORDINATE THESE DRAWINGS WITH EXISTING CONDITIONS, AND COORDINATE ALL DIMENSIONS AND WALL LOCATIONS WITH THE ARCHITECTURAL DRAWINGS. THE GENERAL CONTRACTOR SHALL IMMEDIATELY NOTIFY THE ARCHITECT AND THE STRUCTURAL ENGINEER OF ANY DISCREPANCIES WITHIN THE CONSTRUCTION DOCUMENTS.
2. THE STRUCTURAL DRAWINGS SHOULD NOT BE USED TO SIZE OR LOCATE DOORS, WINDOWS, TOILET PARTITIONS, OR NON-LOAD BEARING WALLS.
3. SEE ARCHITECTURAL FOR ALL EXPANSION JOINT COVERS.
4. DESIGN AND CONSTRUCTION SHALL BE IN ACCORDANCE WITH THE 2018 INTERNATIONAL BUILDING CODE, WITH GEORGIA AMENDMENTS.
5. DESIGN LOADS: FOUNDATION DESIGNED BASED ON COLUMN REACTIONS BY BUTLER BUILDING MANUFACTURER.
6. ALL THE SAFETY REGULATIONS, METHODS OF CONSTRUCTION AND ERECTION OF STRUCTURAL MATERIAL SHALL BE THE RESPONSIBILITY OF THE GENERAL CONTRACTOR. IT SHALL BE THE GENERAL CONTRACTOR'S RESPONSIBILITY TO PROVIDE ADEQUATE SHORING, BRACING, AND FRAMEWORK, ETC. AS REQUIRED.
7. DIMENSIONS ARE NOT TO BE DERIVED BY SCALING THESE DRAWINGS. IF THERE IS ANY QUESTION ABOUT DETAILS OR DIMENSIONS, CONTACT THE ARCHITECT AND STRUCTURAL ENGINEER FOR CLARIFICATION.
8. WHERE A DETAIL IS SHOWN FOR ONE CONDITION, IT SHALL ALSO APPLY FOR ALL LIKE OR SIMILAR CONDITIONS, UNLESS NOTED OTHERWISE.
9. ISOMETRIC VIEWS ARE FOR ILLUSTRATIVE PURPOSES ONLY. NO INFORMATION ABOUT THE STRUCTURE OR ITS COMPONENTS SHALL BE TAKEN OR ASSUMED FROM THEM.
10. CONTRACTOR SHALL TAMP THE VIRGIN SOIL AFTER EXCAVATION UNTIL NO VISIBLE SOIL RUTTING OCCURS FOR THE FULL SIZE OF THE FOOTING. IF SOIL TAMPING DOES NOT COMPACT SOIL TO THIS CRITERIA, THE ENGINEER SHALL BE NOTIFIED TO DETERMINE SOIL REMEDIATION REQUIREMENTS.

REINFORCING STEEL

1. REINFORCING STEEL SHALL BE NEW BILLET STEEL, DEFORMED BARS CONFORMING TO ASTM A615, GRADE 60, AND SHALL BE FREE FROM ANY FORM RELEASE AGENTS.
2. REINFORCING BENDS SHALL CONFORM TO CRSI.
3. WELDED WIRE FABRIC SHALL BE SHEETS OF NEW BILLET STEEL COLD DRAWN, CONFORMING TO ASTM SPECIFICATION A185, GRADE 60.
4. REINFORCING IS TO BE SUPPORTED AND SPACED WITH WIRE BAR SUPPORTS ACCORDING TO CRSI "PLACING REINFORCING BARS" UNLESS NOTED OTHERWISE.
5. BAR SUPPORTS, DESIGN, DETAILING, FABRICATION AND PLACING OF REINFORCING STEEL SHALL BE IN ACCORDANCE WITH ACI 318 AND "THE MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES," ACI 315.
6. SPLICES FOR CONTINUOUS BARS SHALL BE CLASS B, UNLESS NOTED OTHERWISE. WELDED WIRE FABRIC SHALL BE LAPPED 12" MINIMUM.
7. PROVIDE BENT HORIZONTAL BARS AT CORNERS AND INTERSECTIONS OF ALL WALLS AND FOOTINGS. BENT BARS ARE TO MATCH THE SIZE AND SPACING OF HORIZONTAL BARS IN WALL OR FOOTING. USE CLASS B SPLICE EACH SIDE.
8. PROVIDE DIAGONAL BARS AT CORNERS OF OPENINGS IN SLABS AND CONCRETE WALLS. SEE DETAILS "RECTANGULAR OPENING WALL SLAB" AND "CIRCLE OPENING WALL SLAB", PROVIDE 2" CLEAR COVER BETWEEN THE OPENING AND THE CORNER REINFORCING BARS.
9. WALL FOOTING REINFORCEMENT SHALL BE CONTINUOUS THROUGH COLUMN FOOTING.
10. EXTEND ALL FOOTING REINFORCEMENT TO FAR SIDE OF FOOTING. SEE NOTE BELOW FOR CONCRETE COVERAGE.
11. PROVIDE DOWELS IN WALL FOOTING TO MATCH WALL VERTICALS UNLESS NOTED OTHERWISE ON DRAWINGS. PROVIDE CLASS B SPLICE. USE STANDARD ACI 90 DEGREE HOOK WITH 3" CLEAR TO BOTTOM OF FOOTING UNLESS NOTED OTHERWISE. SEE DETAIL "CORNER BAR & SPLICE LENGTH DETAIL (IN CONCRETE)"

CLASS B SPLICE OR CORNER BAR PER ACI 318

BAR #	3000 PSI CONCRETE		4000 PSI CONCRETE		5000 PSI CONCRETE	
	MIN. SPLICE (INCHES)	MIN. SPLICE (BAR DIAM.)	MIN. SPLICE (INCHES)	MIN. SPLICE (BAR DIAM.)	MIN. SPLICE (INCHES)	MIN. SPLICE (BAR DIAM.)
4	29		25		24	
5	36	57	31	50	28	45
6	43		37		34	
7	63		54		49	
8	72		62		56	
9	81	72	70	62	63	56
10	89		78		69	
11	98		85		76	

12. MINIMUM CONCRETE COVERAGE SHALL BE AS FOLLOWS. IF CONSTRUCTION DOCUMENTS INDICATE A LARGER COVERAGE, IT SHALL BE USED. IF STIRRUPS, TIES, OR SPIRALS ARE USED, COVERAGE SHALL BE TO THE OUTERMOST FACE OF THESE ELEMENTS.
    A. FOOTINGS, CAISSONS, AND OTHER MEMBERS WHERE CONCRETE IS DEPOSITED AGAINST SOIL (EXCEPT SLABS ON GRADE) = 3"
    B. CONCRETE EXPOSED TO WEATHER OR SOIL BUT IS NOT DEPOSITED AGAINST SOIL:
        #6 BAR AND LARGER = 2"
        #5 BAR AND SMALLER = 1 1/2"
    C. CONCRETE NOT EXPOSED TO WEATHER OR SOIL:
        SLABS, WALLS, JOISTS #14 BAR AND LARGER = 1 1/2"
        SLABS, WALLS, JOISTS #11 BAR AND SMALLER = 3/4"
        BEAMS AND COLUMNS = 1 1/2"

CONCRETE

1. ALL CONCRETE WORK TO BE DONE IN ACCORDANCE WITH THE CODE REFERENCED EDITION OF ACI-318: "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE"
2. CONCRETE MIX DESIGN REQUIREMENTS AND COMPRESSIVE STRENGTH AT 28 DAYS.

DESCRIPTION	28 DAY STRENGTH (PSI)	WEIGHT PER CUBIC FOOT (PCF)	SLUMP AT POINT OF PLACEMENT	AGGREGATE	FIBERMESH OR WWM
FOOTING AND FOUNDATION WALLS	4000	145	4" +/- 1"	ASTM C33	NONE
SLAB ON GRADE	4000	145	4" +/- 1"	ASTM C33	FIBERMESH @ 1.5LB PER CUBIC YARD OF CONC.

FLY ASH SHALL NOT BE USED. WATER REDUCING ADMIXTURES MAY BE USED TO ACHIEVE SLUMP REQUIREMENTS.

3. SEE ARCHITECTURAL DOCUMENTS FOR JOINT SIZES AND FILLER MATERIALS.
4. LOCATION OF ALL CONSTRUCTION JOINTS, EXCLUDING SLABS ON GRADE, SHALL BE COORDINATED WITH STRUCTURAL ENGINEER.
5. SHOP DRAWINGS SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER SHOWING PROPOSED LOCATIONS OF ANY MATERIAL SUCH AS BUT NOT LIMITED TOO CONDUITS, EMBEDMENTS, OR FIXTURES TO BE PLACED INSIDE ANY STRUCTURAL CONCRETE MEMBER SUCH AS BEAMS, WALLS, SLABS, COLUMNS OR FOOTINGS. THIS IS NOT REQUIRED FOR SLABS ON GRADE OF 4" OR LESS IN THICKNESS.
6. CONCRETE SLAB FLATNESS AND LEVELNESS TOLERANCES SHALL BE IN CONFORMANCE WITH ACI 117, AND SHALL BE SPECIFIED BY THE OWNER UNLESS SUPERSEDED BY THE OWNER'S CRITERIA, CONFORM TO THE FOLLOWING MINIMUM REQUIREMENTS:
    A. PROVIDE A FLOOR SURFACE WHICH IS TRUE AND LEVEL AND ACHIEVES "F NUMBERS" OF FF = 30 AND FL = 20 MINIMUM OVERALL COMPOSITE AND FF = 20 AND FL = 15 MINIMUM AT ANY INDIVIDUAL SECTION WHEN TESTED IN ACCORDANCE WITH ASTM E1155. REMOVE SURFACE IRREGULARITIES TO PROVIDE A CONTINUOUS SMOOTH FINISH.
    B. ALL INTERIOR SLABS TO RECEIVE A SMOOTH TROWEL FINISH UNLESS NOTED.
7. UNLESS SPECIFIED OTHERWISE IN THE SPECIFICATION, TESTING OF CONCRETE SHALL BE IN CONFORMANCE WITH THE REQUIREMENTS OF ACI 318 SECTION 5.6 "EVALUATION AND ACCEPTANCE OF CONCRETE".
8. CONSTRUCTION JOINTS (CNJT.) ARE TO BE LOCATED ON THE THRESHOLD SIDE OF A WALL.
9. SEE DETAIL "TYPICAL SLAB ON GRADE JOINT LAYOUT".
10. THE FOLLOWING PROCEDURES SHALL MEET THE REQUIREMENTS OF THE REFERENCED CODE SECTIONS:

PROCEDURE	REFERENCE SECTION
PREPARATION	ACI 304 - "GUIDE FOR MEASURING, MIXING, TRANSPORTING AND PLACING CONCRETE"
CONVEYING	ACI 318 SECTION 5.3 - "CONVEYING"
DEPOSITING	ACI 318 SECTION 5.10 - "DEPOSITING"
CONSOLIDATION	ACI 309 - "GUIDE FOR CONSOLIDATION OF CONCRETE"
CURING	ACI 308 - "STANDARD PRACTICE FOR CURING CONCRETE"
HOT WEATHER CONCRETING	ACI 305 - "HOT WEATHER CONCRETING"
COLD WEATHER CONCRETING	ACI 308 "COLD WEATHER CONCRETING"

FOUNDATIONS

1. THE FOUNDATION IS DESIGNED USING AN ASSUMED ALLOWABLE SOIL BEARING CAPACITY OF 2000 PSF BASED ON 2024 IBC SECTION 1806.2. IF THE BEARING CONDITIONS VARY FROM WHAT IS SHOWN, OR IF THE SOIL BEARING CAPACITY IS QUESTIONABLE, THE ARCHITECT AND STRUCTURAL ENGINEER ARE TO BE NOTIFIED IMMEDIATELY.
2. ALL BUILDING AREAS SHALL BE COMPACTED TO 98% OF MAXIMUM DRY DENSITY AT OPTIMUM OPTIMUM MOISTURE CONTENT AS DETERMINED IN ACCORDANCE WITH ASTM D698, CURRENT EDITION.
3. A REGISTERED GEOTECHNICAL ENGINEER REPRESENTING THE OWNER SHALL BE PRESENT TO MONITOR COMPACTION AND SETTLEMENT AND VERIFY THE BEARING CAPACITY. ALL WORK SHALL BE PERFORMED IN ACCORDANCE WITH THE RECOMMENDATIONS OF THE GEOTECHNICAL REPORT AND ON-SITE GEOTECHNICAL ENGINEER.
4. REMOVE ALL TOPSOIL, ROOT SYSTEM OR OTHER DELETERIOUS MATERIAL UNDER PROPOSED SLAB AND COLUMN FOOTINGS AND REPLACE WITH SUITABLE COMPACTED FILL OR CRUSHED STONE. STRUCTURAL ENGINEER'S DECISION ON QUESTIONABLE MATERIAL SHALL BE FINAL.
5. BACKFILLING SHALL BE PERFORMED IN EQUAL LIFTS AROUND THE BUILDING PERIMETER TO BALANCE LATERAL EARTH PRESSURE ON THE BUILDING. WALK BEHIND COMPACTION EQUIPMENT IS REQUIRED WITHIN A DISTANCE OF TWO TIMES THE WALL HEIGHT.
6. BACKFILL AGAINST STRUCTURAL WALLS SHALL NOT BE PERFORMED UNTIL WALL AND SLAB ON GRADE HAS OBTAINED SPECIFIED STRENGTH.
7. IF REQUIRED BY THE GEOTECHNICAL REPORT OR THE ON-SITE GEOTECHNICAL ENGINEER, THE GROUND WATER TABLE SHALL BE LOWERED.
8. ALL FOOTINGS TO BE CENTERED UNDER THE COLUMNS OR WALLS THEY SUPPORT, UNLESS NOTED OTHERWISE ON THE DRAWING.
9. UTILITY LINES SHALL NOT BE PLACED THROUGH OR BELOW FOUNDATIONS WITHOUT THE STRUCTURAL ENGINEER'S APPROVAL IN WRITING. THE CONTRACTOR SHALL LOCATE ANY EXISTING UNDERGROUND UTILITIES PRIOR TO ANY CONSTRUCTION.
10. INSPECTIONS BY GEOTECH FIRM ARE REQUIRED FOR EXISTING SOILS CONDITIONS, FILL PLACEMENT, AND LOAD BEARING REQUIREMENTS:
    A. SITE PREPARATION: PRIOR TO PLACEMENT OF PREPARED FILL, THE INSPECTOR SHALL DETERMINE THAT THE SITE HAS BEEN PREPARED IN ACCORDANCE WITH THE ABOVE-REFERENCED GEOTECHNICAL REPORT.
    B. FILL PLACEMENT: DURING PLACEMENT AND COMPACTION OF FILL MATERIAL, THE INSPECTOR SHALL DETERMINE THAT THE PROPER FILL MATERIAL IS BEING USED AND THAT THE MAXIMUM LIFT THICKNESS IS FOLLOWED IN ACCORDANCE WITH THE ABOVE-REFERENCED GEOTECHNICAL REPORT.
    C. EVALUATION OF IN-PLACE DENSITY: THE INSPECTOR SHALL DETERMINE, AT THE FREQUENCIES DETERMINED IN THE SOILS REPORT AND PROJECT SPECIFICATIONS, THAT THE IN-PLACE DRY DENSITY OF THE COMPACTED FILL COMPLIES WITH THE ABOVE-REFERENCED GEOTECHNICAL REPORT.

STRUCTURAL MASONRY (SEISMIC DESIGN CATEGORY C)

1. ALL MASONRY WORK TO BE DONE IN ACCORDANCE WITH THE CODE-REFERENCED EDITION OF ACI-530 "BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES", AND ACI-530.1 "SPECIFICATION FOR MASONRY STRUCTURES."
2. MORTAR SHALL BE PORTLAND CEMENT-LIME AND CONFORM TO ASTM C270.
3. CLAY MASONRY UNITS SHALL HAVE TYPE N MORTAR, NET AREA COMPRESSIVE STRENGTH OF UNITS SHALL BE 6,000 PSI. NET AREA COMPRESSIVE STRENGTH OF INSTALLED MASONRY (FM) SHALL BE 2,000 PSI. REFER TO ARCHITECTURAL DOCUMENTS FOR ASTM DESIGNATIONS.
4. CONCRETE MASONRY UNITS ABOVE AND BELOW GRADE SHALL HAVE TYPE S MORTAR, NET AREA COMPRESSIVE STRENGTH OF UNITS SHALL BE 1,900 PSI, NET AREA COMPRESSIVE STRENGTH OF INSTALLED MASONRY (FM) SHALL BE 1,500 PSI. CONCRETE MASONRY UNITS SHALL CONFORM TO ASTM C90.
5. BLOCK FILL FOR REINFORCED MASONRY SHALL BE FINE GROUT IN CONFORMANCE WITH ASTM C476 MINIMUM COMPRESSIVE STRENGTH METHOD. GROUT SHALL BE 2,000 PSI AT 28 DAYS. GROUT SLUMP SHALL BE 8 TO 11 INCHES.
6. REINFORCING: ASTM A615 - GRADE 60. SEE TABLE BELOW FOR MINIMUM LAP SPLICE LENGTH AND EMBEDMENT OF REINFORCING BARS.

MASONRY REINFORCING LAP SPLICES AND EMBED LENGTH

BAR SIZE	LAP SPLICE LENGTH (IN.)	EMBEDMENT (IN.)
#4	24	18
#5	30	24
#6	36	28
#7	42	32
#8	48	36
#9	54	42

7. MASONRY PREPARATION, CONSTRUCTION AND PROTECTION IN HOT OR COLD WEATHER (GREATER THAN 90 DEGREES FAHRENHEIT OR LESS THAN 40 DEGREES FAHRENHEIT) SHALL BE IN CONFORMANCE WITH ACI 530.1 SECTION 1.8.
8. EMBEDDED CONDUITS, PIPES AND SLEEVES SHALL BE COMPATIBLE WITH MASONRY AND SHALL NOT BE LOCATED IN GROUTED CELLS. PIPES CONTAINING WATER SUBJECT TO FREEZING, MATERIALS IN EXCESS OF 150 DEGREE FAHRENHEIT OR PIPES UNDER PRESSURE IN EXCESS OF 55 PSI SHALL NOT BE EMBEDDED IN MASONRY. GENERAL CONTRACTOR SHALL COORDINATE THE LOCATION OF ALL EMBEDDED ITEMS WITH THE STRUCTURAL ENGINEER PRIOR TO CONSTRUCTION.
9. ALL NON-LOAD BEARING, NON-SHEAR WALLS SHALL BE LATERALLY BRACED AT OR NEAR THE TOP OF THE WALL AT A SPACING NOT TO EXCEED 8'-0" ON CENTER ALONG THE LENGTH OF THE WALL. THE METHOD OF BRACING SHALL BE THE RESPONSIBILITY OF THE GENERAL CONTRACTOR. A WALL MAY BE CONSIDERED BRACED WHERE THE WALLS JOINS ANOTHER NON-LOAD BEARING, NON SHEAR WALL AT CORNERS AND T'S."
10. ALL NON-LOAD BEARING, NON-SHEAR WALLS SHALL NOT EXCEED THE UNBRACED HEIGHT AS SHOWN IN THE TABLE BELOW. THAT IS, IF THE WALLS ARE TALLER THAN THE LIMITING HEIGHT, THEY SHALL BE BRACED EVERY 8'-0" ON CENTER ALONG THE LENGTH OF THE WALL AT OR BELOW THE LIMITING HEIGHT.

CMU LIMITING UNBRACED HEIGHT (FT.)	
6" CMU	18' - 0"
8" CMU	24' - 0"
12" CMU	36' - 0"

THE ALTERNATIVE TO BRACING AS DESCRIBED ABOVE IS TO REINFORCE THE WALL AND BRACE AT OR NEAR THE TOP OF THE WALL AT A SPACING NOT TO EXCEED 8'-0" ON CENTER ALONG THE LENGTH OF THE WALL. IF THE ALTERNATIVE IS CHOSEN, THE SIZE AND SPACING OF REINFORCING WILL BE DETERMINED BY THE STRUCTURAL ENGINEER ON A CASE BY CASE BASIS.

11. ALL MASONRY WALLS SHALL BE REINFORCED WITH HORIZONTAL JOINT REINFORCEMENT @ 16" O.C. VERTICALLY AS FOLLOWS. THE SPACE BETWEEN THESE WIRES SHALL BE THE WIDEST THAT THE MORTAR JOINT WILL ACCOMMODATE.

CMU SIZE	HORIZONTAL JOINT REINFORCING	
	RUNNING BOND	STACK BOND
4"	(1) WL1 @ 16" O.C.	(1) WL1 @ 16" O.C.
6" OR 8"	(2) WL1 @ 16" O.C.	(2) WL1 @ 16" O.C.
12"	(2) WL1 @ 16" O.C.	(2) WL2.8 @ 16" O.C.

12. VERTICAL REBAR SHALL BE CENTERED IN BLOCK WALLS U.N.O.
13. MASONRY CELLS ON WHICH STEEL OR MASONRY LINTELS BEAR MUST BE REINFORCED WITH SAME REINFORCING AS WALL AND MUST BE FILLED WITH GROUT. IF NO WALL REINFORCING IS SHOWN A #4 BAR SHALL BE ASSUMED.
14. PROVIDE HORIZONTAL BOND BEAMS IN ALL MASONRY WALLS AT 10'-0" ON CENTER MAX VERTICALLY AND WITHIN 16" OF THE TOP OF THE WALL. REINFORCING IN BOND BEAMS SHALL BE 1#4 X CONTINUOUS FOR 6" CMU, 1#5 X CONTINUOUS FOR 8" CMU, AND 1#8 X CONTINUOUS FOR 12" CMU.
15. THE MINIMUM LEVEL OF QUALITY ASSURANCE BASED ON ACI 530-11 SHALL BE LEVEL C. REFER TO TABLE 1.15.3 IN ACI 530-11.
16. WHERE CONCRETE BLOCK IS ABOVE AND BELOW EQUIPMENT PLATFORM, FIT BLOCK TIGHT TO UNDERSIDE OF BEAM PRIOR TO CONSTRUCTING BLOCK ABOVE.
17. MASONRY CELLS SHALL BE GROUTED ROUTINELY TO MINIMIZE GROUT FALL HEIGHT TO A MAXIMUM OF 4'-0".

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GEORGIA REGISTERED No. PE011263 PROFESSIONAL ENGINEER
1-30-2026

NEW WAREHOUSE
4090 SAN CARLOS DRIVE
MACON, GA 31206

STROUD AND COMPANY

MARK	DATE	DESCRIPTION

SHEET TITLE
General Notes

PROJECT DATE: X
PROJECT NUMBER: X
DRAWN BY: DWW

S0.0

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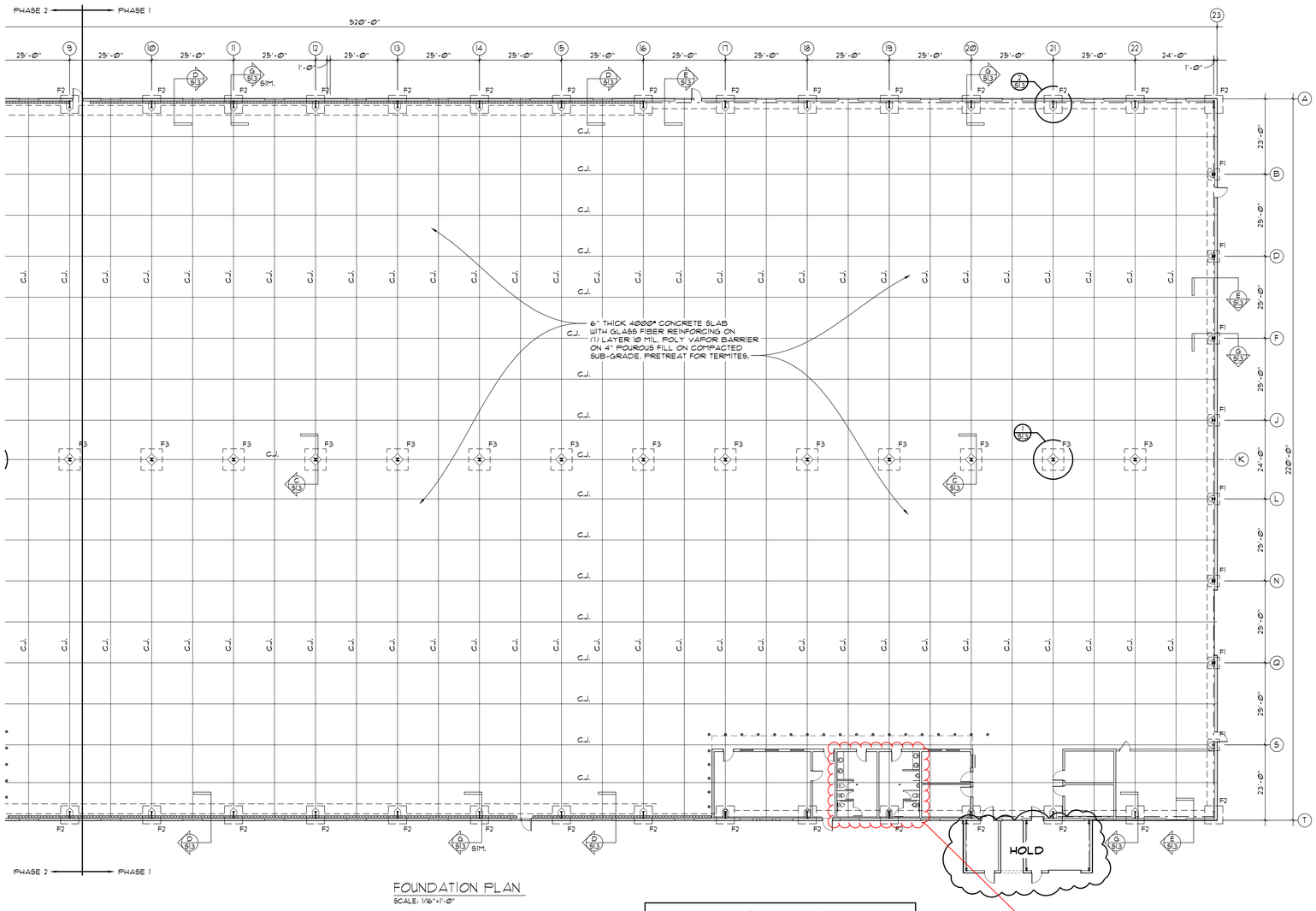

FOUNDATION PLAN

SCALE: 1/16"=1'-0"

FOOTING SCHEDULE:

FOOTING SIZES ARE APPROXIMATE ONLY PENDING REVIEW OF FINAL P.E.M.B. REACTIONS. FOR BIDDING PURPOSES ONLY - "NOT FOR CONSTRUCTION".

ALL BASE PLATES RECESS 8" EXCEPT AT FOOTINGS F4 AND F24

F1	3'-6"x3'-6"x2'-0" DP W/ 2 MATTS (4) #5 REBAR E.W. (1) 3" FROM BOT. AND (1) 3" FROM TOP
F2	5'-6"x5'-6"x2'-0" DP W/ 2 MATTS (5) #6 REBAR E.W. (1) 3" FROM BOT. AND (1) 3" FROM TOP
F3	6'-6"x6'-6"x2'-6" DP W/ 2 MATTS (6) #6 REBAR E.W. (1) 3" FROM BOT. AND (1) 3" FROM TOP
F4	5'-0"x5'-0"x2'-0" DP W/ 2 MATTS (6) #6 REBAR E.W. (1) 3" FROM BOT. AND (1) 3" FROM TOP

Leave slab out until plumbing is designed and permitted

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1-30-2026

NEW WAREHOUSE

4090 SAN CARLOS DRIVE
MACON, GA
31206

STROUD AND COMPANY

MARK	DATE	DESCRIPTION

SHEET TITLE

Foundation Plan Phase 1

PROJECT DATE: X

PROJECT NUMBER: X

DRAWN BY: DWW

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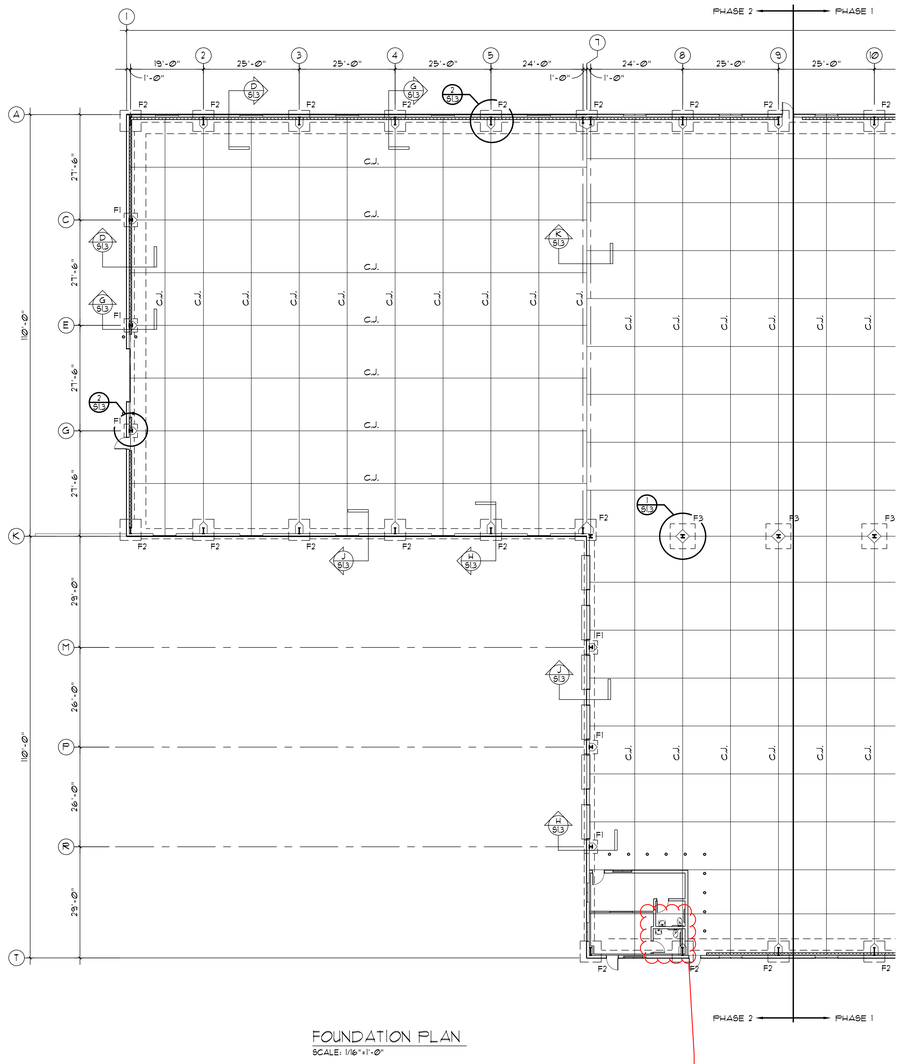
S1.1



FOUNDATION PLAN

SCALE: 1/16"=1'-0"

Leave slab out until plumbing is designed and permitted.

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1-30-2026

NEW WAREHOUSE

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STRONG AND COMPANY

MARK	DATE	DESCRIPTION

SHEET TITLE

Foundation Plan
Phase 2

PROJECT DATE: X

PROJECT NUMBER: X

DRAWN BY: DWW

S1.2

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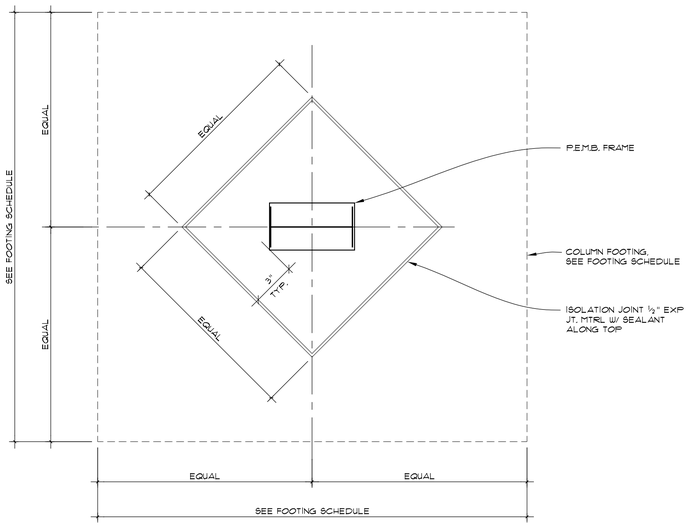
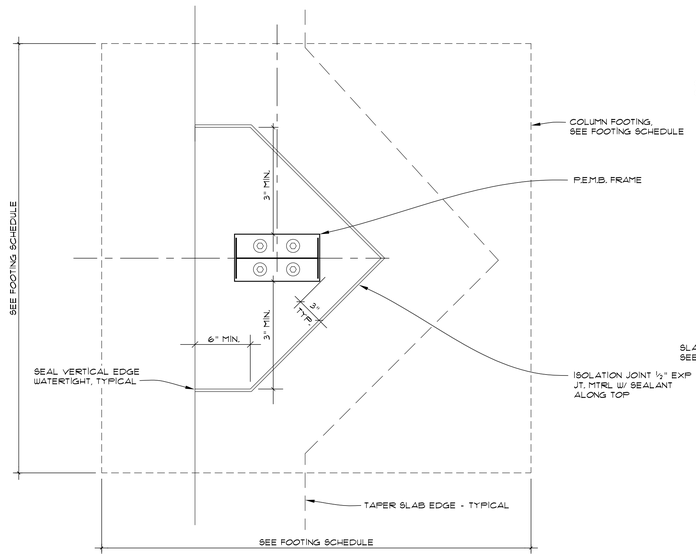
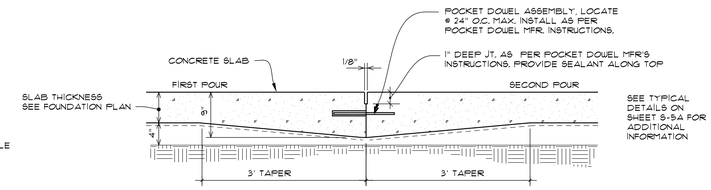


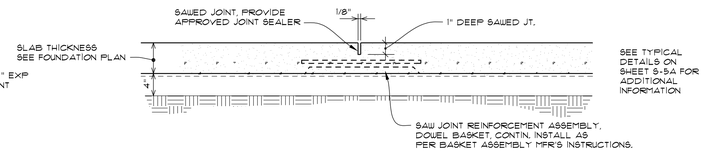
1 S1.3 PLAN - INTERIOR SPREAD FOOTING
SCALE: 1"=1'-0"



2 S1.3 PLAN - EXTERIOR WALL FOOTING
SCALE: 1"=1'-0"

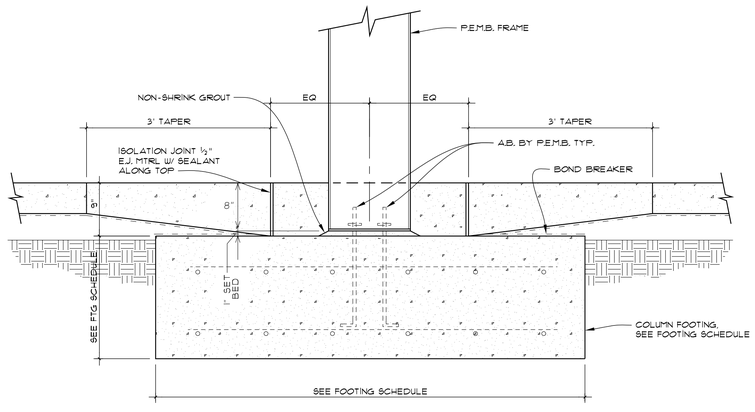


A S1.3 DETAIL - CONSTRUCTION JOINT
SCALE: 1"=1'-0"

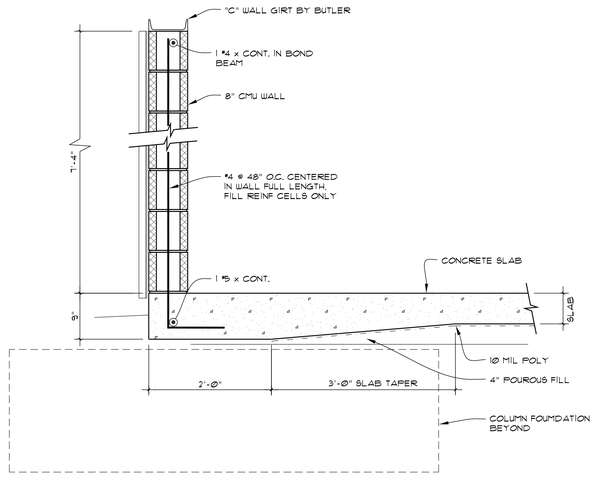


LOCATE SAWED JOINTS @ 12'-6" O.C. MAX. CENTER IN COLUMN GRID/LAYOUT. SAW JOINT IMMEDIATELY AFTER FINAL SET AND AS SOON AS COARSE AGGREGATE WILL NOT BE DISLODGED DURING SAWING PROCESS.

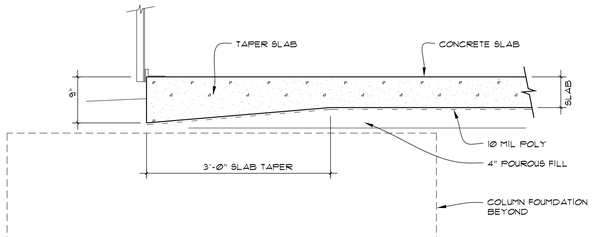
B S1.3 DETAIL - CONTROL JOINT (C.J.)
SCALE: 1"=1'-0"



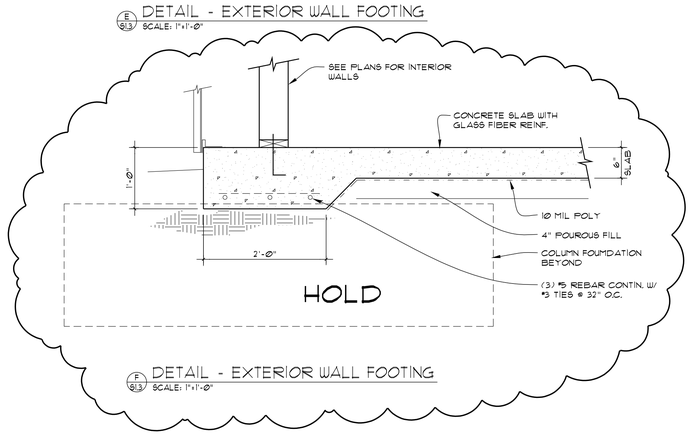
C S1.3 DETAIL - INTERIOR SPREAD FOOTING
SCALE: 1"=1'-0"



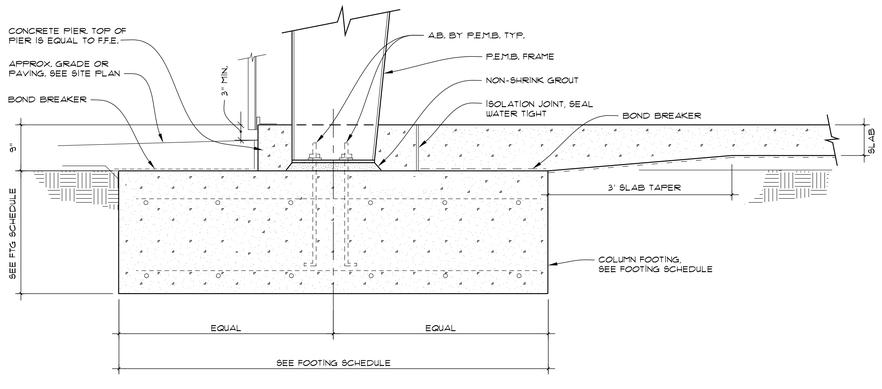
D S1.3 DETAIL - EXTERIOR WALL FOOTING
SCALE: 1"=1'-0"



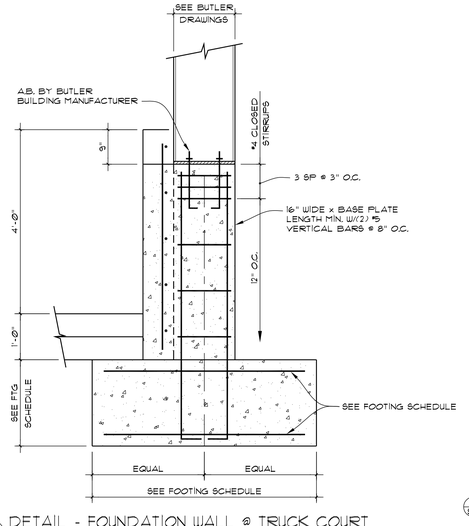
E S1.3 DETAIL - EXTERIOR WALL FOOTING
SCALE: 1"=1'-0"



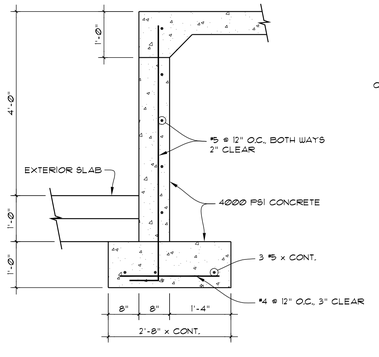
F S1.3 DETAIL - EXTERIOR WALL FOOTING
SCALE: 1"=1'-0"



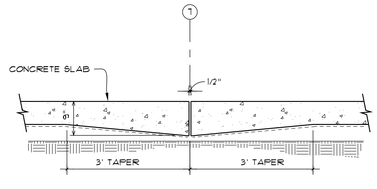
G S1.3 DETAIL - EXTERIOR WALL FOOTING
SCALE: 1"=1'-0"



H S1.3 DETAIL - FOUNDATION WALL @ TRUCK COURT
SCALE: ¾"=1'-0"



J S1.3 DETAIL - FOUNDATION WALL @ TRUCK COURT
SCALE: ¾"=1'-0"



K S1.3 SECTION
SCALE: ¾"=1'-0"

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1-30-2026

NEW WAREHOUSE
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STROUD AND COMPANY

MARK	DATE	DESCRIPTION

SHEET TITLE
Foundation Sections

PROJECT DATE: X
PROJECT NUMBER: X
DRAWN BY: DWW

S1.3

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