GENERAL

1. SEE PROJECT SPECIFICATIONS FOR REQUIREMENTS IN ADDITION TO GENERAL NOTES. COORDINATE THESE DRAWINGS WITH EXISTING CONDITIONS, AND COORDINATE ALL DIMENSIONS AND WALL LOCATIONS WITH THE ARCHITECTURAL DRAWINGS. THE GENERAL CONTRACTOR SHALL IMMEDIATELY NOTIFY THE ARCHITECT AND THE STRUCTURAL ENGINEER OF ANY DISCREPANCIES WITHIN THE CONSTRUCTION DOCUMENTS.

2. THE STRUCTURAL DRAWINGS SHOULD NOT BE USED TO SIZE OR LOCATE DOORS, WINDOWS, TOILET PARTITIONS, OR NON-LOAD BEARING WALLS.

3. SEE ARCHITECTURAL FOR ALL EXPANSION JOINT COVERS.

4. DESIGN AND CONSTRUCTION SHALL BE IN ACCORDANCE WITH THE 2018 INTERNATIONAL BUILDING CODE, WITH GEORGIA AMENDMENTS.

5. DESIGN LOADS:

   LIVE LOAD INFORMATION
   - A. ROOF LIVE LOAD (REDUCED FOR TRIBUTARY AREA) = 20 PSF
   - B. FLOOR LIVE LOAD @ STAIRS = 100 PSF
   - C. FLOOR LIVE LOAD = 100 PSF

   FLOOR DEAD LOADING INFORMATION
   - A. 10 PSF STEEL FRAMING SYSTEM
   - B. 40 PSF CONCRETE AND STEEL DECK
   - C. 6.0 PSF COLLATERAL LOAD (LIGHTS, HVAC, SPRINKLER, ETC.)
   - D. WEIGHT OF WALLS CONTRIBUTING TO DESIGN LOAD VARIES
   - E. SEE FRAMING PLAN FOR OTHER CONCENTRATED LOADS

   ROOF DEAD LOADING INFORMATION
   - A. 10 PSF STEEL FRAMING SYSTEM, DECKING, INSULATION AND ROOFING
   - B. 6.0 PSF COLLATERAL LOAD (LIGHTS, HVAC, SPRINKLER, ETC.)
   - C. SEE FRAMING PLAN FOR OTHER CONCENTRATED LOADS

   SNOW LOAD INFORMATION
   - A. GROUND SNOW LOAD (PG) = 5 PSF
   - B. FLAT-ROOF SNOW LOAD (PF) = 5.5 PSF
   - C. SNOW EXPOSURE FACTOR (CE) = 1.0
   - D. SNOW LOAD IMPORTANCE FACTOR(IS) = 1.1
   - E. THERMAL FACTOR (CT) = 1.0

   WIND LOAD INFORMATION
   - A. ULT. WIND SPEED = 110 MPH
   - B. ASD WIND SPEED = 85 MPH
   - C. WIND IMPORTANCE FACTOR (IW) = 1.0
   - D. RISK CATEGORY = II
   - E. WIND EXPOSURE = C
   - F. INTERNAL PRESSURE COEFFICIENT = +/- 0.18
   - G. COMPONENTS AND CLADDING = SEE SCHEDULE ON SHEET S0.1

   SEISMIC DESIGN INFORMATION
   - A. SEISMIC IMPORTANCE FACTOR (IE) = 1.0
   - B. SEISMIC DESIGN CATEGORY = C
   - C. 0.2 SECOND SPECTRAL RESPONSE ACCELERATION (SS) = 0.178
   - D. 1 SECOND SPECTRAL RESPONSE ACCELERATION (S1) = 0.077
   - E. 0.2 DESIGN SPECTRAL RESPONSE ACCELERATION (SDS) = 0.190
   - F. 1 DESIGN SECOND SPECTRAL RESPONSE ACCELERATION (SD1) = 0.123
   - G. SITE CLASS = D (PER GEOTECHNICAL REPORT)
   - H. RESPONSE MODIFICATION COEFFICIENT (R) = 3.5
   - I. SYSTEM OVERSTRENGTH FACTOR = 3
   - J. DEFLECTION AMPLIFICATION FACTOR (CD) = 4
   - K. SEISMIC RESPONSE COEFFICIENT (CS) = 0.073
   - L. DESIGN BASE SHEAR (VX) = 79 KIPS BOTH WAYS
   - M. BASIC SEISMIC FORCE RESISTING SYSTEM - CMU SHEAR WALLS
   - N. ANALYSIS PROCEDURE - EQUIVALENT LATERAL FORCE PROCEDURE
   - O. SEISMIC RISK CATEGORY = II

   RAIN INTENSITY FACTOR = 4.0 INCHES PER HOUR

6. ALL THE SAFETY REGULATIONS, METHODS OF CONSTRUCTION AND ERECTION OF STRUCTURAL MATERIAL SHALL BE THE RESPONSIBILITY OF THE GENERAL CONTRACTOR. IT SHALL BE THE GENERAL CONTRACTOR'S RESPONSIBILITY TO PROVIDE ADEQUATE SHORING, BRACING, AND FRAMEWORK, ETC. AS REQUIRED.

7. DIMENSIONS ARE NOT TO BE DERIVED BY SCALING THESE DRAWINGS. IF THERE IS ANY QUESTION ABOUT DETAILS OR DIMENSIONS, CONTACT THE ARCHITECT AND STRUCTURAL ENGINEER FOR CLARIFICATION.

8. WHERE A DETAIL IS SHOWN FOR ONE CONDITION, IT SHALL ALSO APPLY FOR ALL LIKE OR SIMILAR CONDITIONS, UNLESS NOTED OTHERWISE.

9. ISOMETRIC VIEWS ARE FOR ILLUSTRATIVE PURPOSES ONLY. NO INFORMATION ABOUT THE STRUCTURE OR ITS COMPONENTS SHALL BE TAKEN OR ASSUMED FROM THEM.

10. CONTRACTOR SHALL TAMP THE VIRGIN SOIL AFTER EXCAVATION UNTIL NO VISIBLE SOIL RUTTING OCCURS FOR THE FULL SIZE OF THE FOOTING. IF SOIL TAMPING DOES NOT COMPACT SOIL TO THIS CRITERIA, THE ENGINEER SHALL BE NOTIFIED TO DETERMINE SOIL REMEDIATION REQUIREMENTS.

SUBMITTALS

1. THE CONTRACT DOCUMENTS ARE THE STRUCTURAL ENGINEER'S INSTRUMENTS OF SERVICE TO CONVEY DESIGN INTENT. THEY ARE NOT TO BE CONSIDERED FABRICATION OR LAYOUT DRAWINGS.

2. THE FOLLOW ARE REQUIRED SUBMITTALS
   - A. CONCRETE MIX DESIGN(S)
   - B. REINFORCING BAR DRAWINGS
   - C. MASONRY MATERIAL CERTIFICATES, ACCESSORIES, AND GROUT MIX DESIGN
   - D. STRUCTURAL STEEL
   - E. METAL DECK
   - F. STEEL JOISTS
   - G. LIGHT GAUGE METAL FRAMING
   - H. OTHER SUBMITTALS AS NOTED ON THE DRAWINGS AND SPECIFICATIONS

3. SUBMITTALS SHALL BE REVIEWED BY THE CONTRACTOR PRIOR TO SUBMISSION TO THE STRUCTURAL ENGINEER AND SHALL BEAR THE CONTRACTOR'S STAMP ATTESTING TO THE DRAWINGS NOT STAMPED WILL NOT BE REVIEWED. SUBCONTRACTOR'S UNCHECKED SUBMITTAL DRAWINGS WILL NOT BE REVIEWED.

4. SUBMITTALS TO BE REVIEWED BY THE STRUCTURAL ENGINEER SHALL BE SUBMITTED TO THE ARCHITECT. THE STRUCTURAL ENGINEER WILL NOT ACCEPT SUBMITTALS DIRECTLY FROM CONTRACTORS WITHOUT THE STRUCTURAL ENGINEER'S PRIOR APPROVAL.

5. UPON COMPLETION OF THE STRUCTURAL ENGINEER'S REVIEW, SUBMITTALS WILL BE RETURNED TO THE ARCHITECT FOR THEIR REVIEW.

6. ANY DEVIATION IN DESIGN, DETAILS, DIMENSIONS, ETC. FROM THE CONSTRUCTION DOCUMENTS SHALL BE CLOUDED ON THE SUBMITTAL AND VERIFICATION OF THE CHANGE SHALL BE REQUESTED.

FOUNDATIONS

1. THE FOUNDATION IS DESIGNED USING AN ASSUMED ALLOWABLE SOIL BEARING CAPACITY OF 2000 PSF BASED ON 2018 IBC SECTION 1806. IF THE BEARING CONDITIONS VARY FROM WHAT IS SHOWN, OR IF THE SOIL BEARING CAPACITY IS QUESTIONABLE, THE ARCHITECT AND STRUCTURAL ENGINEER ARE TO BE NOTIFIED IMMEDIATELY.

2. ALL BUILDING AREAS SHALL BE COMPACTED TO 98% OF MAXIMUM DRY DENSITY AT OPTIMUM OPTIMUM MOISTURE CONTENT AS DETERMINED IN ACCORDANCE WITH ASTM D698, CURRENT EDITION.

3. A REGISTERED GEOTECHNICAL ENGINEER REPRESENTING THE OWNER SHALL BE PRESENT TO MONITOR COMPACTION AND SETTLEMENT AND VERIFY THE BEARING CAPACITY. ALL WORK SHALL BE PERFORMED IN ACCORDANCE WITH THE RECOMMENDATIONS OF THE GEOTECHNICAL REPORT AND ON-SITE GEOTECHNICAL ENGINEER.

4. REMOVE ALL TOPSOIL, ROOT SYSTEM OR OTHER DELETERIOUS MATERIAL UNDER PROPOSED SLAB AND COLUMN FOOTINGS AND REPLACE WITH SUITABLE COMPACTED FILL OR CRUSHED STONE. STRUCTURAL ENGINEER'S DECISION ON QUESTIONABLE MATERIAL SHALL BE FINAL.

5. BACKFILLING SHALL BE PERFORMED IN EQUAL LIFTS AROUND THE BUILDING PERIMETER TO BALANCE LATERAL EARTH PRESSURE ON THE BUILDING. WALK BEHIND COMPACTION EQUIPMENT IS REQUIRED WITHIN A DISTANCE OF TWO TIMES THE WALL HEIGHT.

6. BACKFILL AGAINST STRUCTURAL WALLS SHALL NOT BE PERFORMED UNTIL WALL AND SLAB ON GRADE HAS OBTAINED SPECIFIED STRENGTH.

7. IF REQUIRED BY THE GEOTECHNICAL REPORT OR THE ON-SITE GEOTECHNICAL ENGINEER, THE GROUND WATER TABLE SHALL BE LOWERED.

8. ALL FOOTINGS TO BE CENTERED UNDER THE COLUMNS OR WALLS THEY SUPPORT, UNLESS NOTED OTHERWISE ON THE DRAWING.

9. UTILITY LINES SHALL NOT BE PLACED THROUGH OR BELOW FOUNDATIONS WITHOUT THE STRUCTURAL ENGINEER'S APPROVAL IN WRITING. THE CONTRACTOR SHALL LOCATE ANY EXISTING UNDERGROUND UTILITIES PRIOR TO ANY CONSTRUCTION.

10. INSPECTIONS BY GEOTECH FIRM ARE REQUIRED FOR EXISTING SOILS CONDITIONS, FILL PLACEMENT, AND LOAD BEARING REQUIREMENTS:
    - A. SITE PREPARATION: PRIOR TO PLACEMENT OF PREPARED FILL, THE INSPECTOR SHALL DETERMINE THAT THE SITE HAS BEEN PREPARED IN ACCORDANCE WITH THE ABOVE-REFERENCED GEOTECHNICAL REPORT.
    - B. FILL PLACEMENT: DURING PLACEMENT AND COMPACTION OF FILL MATERIAL, THE INSPECTOR SHALL DETERMINE THAT THE PROPER FILL MATERIAL IS BEING USED AND THAT THE MAXIMUM LIFT THICKNESS IS FOLLOWED IN ACCORDANCE WITH THE ABOVE-REFERENCED GEOTECHNICAL REPORT.
    - C. EVALUATION OF IN-PLACE DENSITY: THE INSPECTOR SHALL DETERMINE, AT THE FREQUENCIES DETERMINED IN THE SOILS REPORT AND PROJECT SPECIFICATIONS, THAT THE IN-PLACE DRY DENSITY OF THE COMPACTED FILL COMPLIES WITH THE ABOVE-REFERENCED GEOTECHNICAL REPORT.

CONCRETE

1. ALL CONCRETE WORK TO BE DONE IN ACCORDANCE WITH THE CODE REFERENCED EDITION OF ACI-318: "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE"

2. CONCRETE MIX DESIGN REQUIREMENTS AND COMPRESSIVE STRENGTH AT 28 DAYS.

DESCRIPTION	28 DAY STRENGTH (PSI)	WEIGHT PER CUBIC FOOT (PCF)	SLUMP AT POINT OF PLACEMENT	AGGREGATE	FIBERMESH OR WWM
FOOTING AND FOUNDATION WALLS	4000	145	4" +/- 1"	ASTM C33	NONE
SLAB ON GRADE	4000	145	4" +/- 1"	ASTM C33	FIBERMESH @ 1.5LB PER CUBIC YARD OF CONC.
EXTERIOR SLAB ON GRADE	4500	145	4" +/- 1"	ASTM C33	WWM 6X6 W1.4 X W1.4

FLY ASH SHALL NOT BE USED. WATER REDUCING ADMIXTURES MAY BE USED TO ACHIEVE SLUMP REQUIREMENTS.

3. SEE ARCHITECTURAL DOCUMENTS FOR JOINT SIZES AND FILLER MATERIALS.

4. LOCATION OF ALL CONSTRUCTION JOINTS, EXCLUDING SLABS ON GRADE, SHALL BE COORDINATED WITH STRUCTURAL ENGINEER.

5. SHOP DRAWINGS SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER SHOWING PROPOSED LOCATIONS OF ANY MATERIAL SUCH AS BUT NOT LIMITED TOO CONDUITS, EMBEDMENTS, OR FIXTURES TO BE PLACED INSIDE ANY STRUCTURAL CONCRETE MEMBER SUCH AS BEAMS, WALLS, SLABS, COLUMNS OR FOOTINGS. THIS IS NOT REQUIRED FOR SLABS ON GRADE OF 4" OR LESS IN THICKNESS.

6. CONCRETE SLAB FLATNESS AND LEVELNESS TOLERANCES SHALL BE IN CONFORMANCE WITH ACI 117, AND SHALL BE SPECIFIED BY THE OWNER, UNLESS SUPERSEDED BY THE OWNER'S CRITERIA, CONFORM TO THE FOLLOWING MINIMUM REQUIREMENTS:
    - A. PROVIDE A FLOOR SURFACE WHICH IS TRUE AND LEVEL AND ACHIEVES "F NUMBERS" OF FF = 30 AND FL = 20 MINIMUM OVERALL COMPOSITE AND FF = 20 AND FL = 15 MINIMUM AT ANY INDIVIDUAL SECTION, WHEN TESTED IN ACCORDANCE WITH ASTM E1155, REMOVE SURFACE IRREGULARITIES TO PROVIDE A CONTINUOUS SMOOTH FINISH.
    - B. ALL INTERIOR SLABS TO RECEIVE A SMOOTH TROWEL FINISH UNLESS NOTED.

7. UNLESS SPECIFIED OTHERWISE IN THE SPECIFICATION, TESTING OF CONCRETE SHALL BE IN CONFORMANCE WITH THE REQUIREMENTS OF ACI 318 SECTION 5.6 "EVALUATION AND ACCEPTANCE OF CONCRETE".

8. CONSTRUCTION JOINTS (CNJT.) ARE TO BE LOCATED ON THE THRESHOLD SIDE OF A WALL.

9. SEE DETAIL "TYPICAL SLAB ON GRADE JOINT LAYOUT".

10. THE FOLLOWING PROCEDURES SHALL MEET THE REQUIREMENTS OF THE REFERENCED CODE SECTIONS:

PROCEDURE	REFERENCE SECTION
PREPARATION	ACI 304 - "GUIDE FOR MEASURING, MIXING, TRANSPORTING AND PLACING CONCRETE"
CONVEYING	ACI 318 SECTION 5.9 - "CONVEYING"
DEPOSITING	ACI 318 SECTION 5.10 - "DEPOSITING"
CONSOLIDATION	ACI 309 - "GUIDE FOR CONSOLIDATION OF CONCRETE"
CURING	ACI 308 - "STANDARD PRACTICE FOR CURING CONCRETE"
HOT WEATHER CONCRETING	ACI 305 - "HOT WEATHER CONCRETING"
COLD WEATHER CONCRETING	ACI 306 "COLD WEATHER CONCRETING"

STRUCTURAL MASONRY (SEISMIC DESIGN CATEGORY C)

1. ALL MASONRY WORK TO BE DONE IN ACCORDANCE WITH THE CODE-REFERENCED EDITION OF ACI-530 "BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES", AND ACI-530.1 "SPECIFICATION FOR MASONRY STRUCTURES."

2. MORTAR SHALL BE PORTLAND CEMENT-LIME AND CONFORM TO ASTM C270.

3. CLAY MASONRY UNITS SHALL HAVE TYPE N MORTAR. NET AREA COMPRESSIVE STRENGTH OF UNITS SHALL BE 6200 PSI. NET AREA COMPRESSIVE STRENGTH OF INSTALLED MASONRY (FM) SHALL BE 2000 PSI. REFER TO ARCHITECTURAL DOCUMENTS FOR ASTM DESIGNATIONS.

4. CONCRETE MASONRY UNITS ABOVE AND BELOW GRADE SHALL HAVE TYPE S MORTAR. NET AREA COMPRESSIVE STRENGTH OF UNITS SHALL BE 1900 PSI. NET AREA COMPRESSIVE STRENGTH OF INSTALLED MASONRY (FM) SHALL BE 1500 PSI. CONCRETE MASONRY UNITS SHALL CONFORM TO ASTM C90.

5. BLOCK FILL FOR REINFORCED MASONRY SHALL BE FINE GROUT IN CONFORMANCE WITH ASTM C476 MINIMUM COMPRESSIVE STRENGTH METHOD. GROUT SHALL BE 2000 PSI AT 28 DAYS. GROUT SLUMP SHALL BE 8 TO 11 INCHES.

6. REINFORCING: ASTM A615 - GRADE 60. SEE TABLE BELOW FOR MINIMUM LAP SPLICE LENGTH AND EMBEDMENT OF REINFORCING BARS.

MASONRY REINFORCING LAP SPLICES AND EMBED LENGTH

BAR SIZE	LAP SPLICE LENGTH (IN.)	EMBEDMENT (IN.)
#4	24	18
#5	30	24
#6	36	28
#7	42	32
#8	48	36
#9	54	42

7. MASONRY PREPARATION, CONSTRUCTION AND PROTECTION IN HOT OR COLD WEATHER (GREATER THAN 90 DEGREES FAHRENHEIT OR LESS THAN 40 DEGREES FAHRENHEIT) SHALL BE IN CONFORMANCE WITH ACI 530.1 SECTION 1.8.

8. EMBEDDED CONDUITS, PIPES AND SLEEVES SHALL BE COMPATIBLE WITH MASONRY AND SHALL NOT BE LOCATED IN GROUTED CELLS. PIPES CONTAINING WATER SUBJECT TO FREEZING, MATERIALS IN EXCESS OF 150 DEGREE FAHRENHEIT OR PIPES UNDER PRESSURE IN EXCESS OF 55 PSI SHALL NOT BE EMBEDDED IN MASONRY. GENERAL CONTRACTOR SHALL COORDINATE THE LOCATION OF ALL EMBEDDED ITEMS WITH THE STRUCTURAL ENGINEER PRIOR TO CONSTRUCTION.

9. ALL NON-LOAD BEARING, NON-SHEAR WALLS SHALL BE LATERALLY BRACED AT OR NEAR THE TOP OF THE WALL AT A SPACING NOT TO EXCEED 8'-0" ON CENTER ALONG THE LENGTH OF THE WALL. THE METHOD OF BRACING SHALL BE THE RESPONSIBILITY OF THE GENERAL CONTRACTOR. A WALL MAY BE CONSIDERED BRACED WHERE THE WALLS JOINS ANOTHER NON-LOAD BEARING, NON SHEAR WALL AT CORNERS AND "T'S."

10. ALL NON-LOAD BEARING, NON-SHEAR WALLS SHALL NOT EXCEED THE UNBRACED HEIGHT AS SHOWN IN THE TABLE BELOW. THAT IS, IF THE WALLS ARE TALLER THAN THE LIMITING HEIGHT, THEY SHALL BE BRACED EVERY 8'-0" ON CENTER ALONG THE LENGTH OF THE WALL AT OR BELOW THE LIMITING HEIGHT.

CMU LIMITING UNBRACED HEIGHT (FT)	
6" CMU	18' - 0"
8" CMU	24' - 0"
12" CMU	36' - 0"

THE ALTERNATIVE TO BRACING AS DESCRIBED ABOVE IS TO REINFORCE THE WALL AND BRACE AT OR NEAR THE TOP OF THE WALL AT A SPACING NOT TO EXCEED 8'-0" ON CENTER ALONG THE LENGTH OF THE WALL. IF THE ALTERNATIVE IS CHOSEN, THE SIZE AND SPACING OF REINFORCING WILL BE DETERMINED BY THE STRUCTURAL ENGINEER ON A CASE BY CASE BASIS.

11. ALL MASONRY WALLS SHALL BE REINFORCED WITH HORIZONTAL JOINT REINFORCEMENT @ 16" O.C. VERTICALLY AS FOLLOWS, THE SPACE BETWEEN THESE WIRES SHALL BE THE WIDEST THAT THE MORTAR JOINT WILL ACCOMMODATE.

CMU SIZE	HORIZONTAL JOINT REINFORCING	
	RUNNING BOND	STACK BOND
4"	(1) W1.7 @ 16" O.C.	(1) W1.7 @ 16" O.C.
6" OR 8"	(2) W1.7 @ 16" O.C.	(2) W1.7 @ 16" O.C.
12"	(2) W1.7 @ 16" O.C.	(2) W2.8 @ 16" O.C.

12. VERTICAL REBAR SHALL BE CENTERED IN BLOCK WALLS U.N.O.

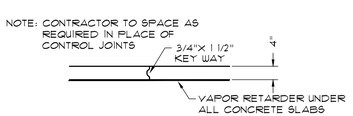
13. MASONRY CELLS ON WHICH STEEL OR MASONRY LINTELS BEAR MUST BE REINFORCED WITH SAME REINFORCING AS WALL AND MUST BE FILLED WITH GROUT. IF NO WALL REINFORCING IS SHOWN A #4 BAR SHALL BE ASSUMED.

14. PROVIDE HORIZONTAL BOND BEAMS IN ALL MASONRY WALLS AT 10'-0" ON CENTER MAX VERTICALLY AND WITHIN 16" OF THE TOP OF THE WALL. REINFORCING IN BOND BEAMS SHALL BE 1#4 X CONTINUOUS FOR 6" CMU, 1#5 X CONTINUOUS FOR 8" CMU, AND 1#6 X CONTINUOUS FOR 12" CMU.

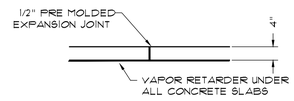
15. THE MINIMUM LEVEL OF QUALITY ASSURANCE BASED ON ACI 530-11 SHALL BE LEVEL C. REFER TO TABLE 1.15.3 IN ACI 530-11.

16. WHERE CONCRETE BLOCK IS ABOVE AND BELOW EQUIPMENT PLATFORM, FIT BLOCK TIGHT TO UNDERSIDE OF BEAM PRIOR TO CONSTRUCTING BLOCK ABOVE.

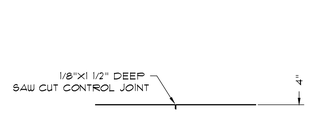
17. MASONRY CELLS SHALL BE GROUTED ROUTINELY TO MINIMIZE GROUT FALL HEIGHT TO A MAXIMUM OF 4'-0".



TYPICAL CONSTRUCTION JOINTS (CNJT.)
N.T.S.



TYPICAL ISOLATION JOINT (I. JT.)
N.T.S.



TYPICAL CONTROL JOINT (C.J.)
N.T.S.

SAW CUTTING CONTROL JOINTS IS AN ATTEMPT TO PARTIALLY CONTROL THE SHRINKAGE CRACKS THAT NATURALLY OCCURS IN CONCRETE DURING THE CURING PROCESS. SOMETIMES THE CONCRETE WILL CRACK BETWEEN CONTROL JOINTS.

Kornegay Engineering Inc.
Structural Consultant
363 Pierce Avenue
Suite 202
Macon, GA 31204
(478)745-6161 ph
Project No:24-311

GEORGIA REGISTERED No. PE011263 PROFESSIONAL ENGINEER
5-16-2025

DUNWODY/BEELAND, Architects, Inc.
300 Mulberry Street, Suite 604
Macon, GA 31201
T 478.742.5321
F 478.743.0863
www.dunwodybeeland.com

OFFICE RENOVATION FOR VILTIES HOLDINGS LLC

4951 FORSYTH ROAD, MACON, GA 31210

Revisions:	

Sheet Title:

GENERAL NOTES

Project #: 2229

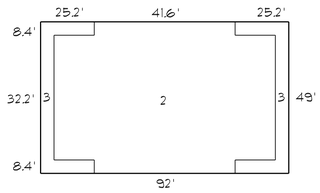
Date: 3/7/2025

S0.0

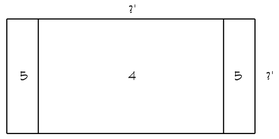
COMPONENTS AND CLADDING

ZONE	≤ 10 ft²	50 ft²	100 ft²	200 ft²	500 ft²
1	---	---	---	---	---
2	+17.0 -67.3	---	+17.0 -52.9	+17.0 -48.6	+17.0 -42.9
3	+17.0 -91.8	---	+17.0 -63.0	+17.0 -54.3	+17.0 -42.9
4	+29.3 -31.8	+26.3 -28.7	---	+23.7 -26.1	+22.0 -24.4
5	+29.3 -39.1	+26.3 -33.1	---	+23.7 -27.9	+22.0 -24.4

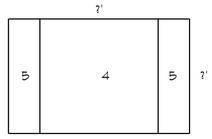
ROOF ZONES



WALL ZONES



WALL ZONES



REINFORCING STEEL

1. REINFORCING STEEL SHALL BE NEW BILLET STEEL, DEFORMED BARS CONFORMING TO ASTM A615, GRADE 60, AND SHALL BE FREE FROM ANY FORM RELEASE AGENTS.
2. REINFORCING BENDS SHALL CONFORM TO CRSI.
3. WELDED WIRE FABRIC SHALL BE SHEETS OF NEW BILLET STEEL COLD DRAWN, CONFORMING TO ASTM SPECIFICATION A185, GRADE 60.
4. REINFORCING IS TO BE SUPPORTED AND SPACED WITH WIRE BAR SUPPORTS ACCORDING TO CRSI "PLACING REINFORCING BARS" UNLESS NOTED OTHERWISE.
5. BAR SUPPORTS, DESIGN, DETAILING, FABRICATION AND PLACING OF REINFORCING STEEL SHALL BE IN ACCORDANCE WITH ACI 318 AND "THE MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES," ACI 315.
6. SPLICES FOR CONTINUOUS BARS SHALL BE CLASS B, UNLESS NOTED OTHERWISE. WELDED WIRE FABRIC SHALL BE LAPPED 12" MINIMUM.
7. PROVIDE BENT HORIZONTAL BARS AT CORNERS AND INTERSECTIONS OF ALL WALLS AND FOOTINGS. BENT BARS ARE TO MATCH THE SIZE AND SPACING OF HORIZONTAL BARS IN WALL OR FOOTING. USE CLASS B SPLICE EACH SIDE.
8. PROVIDE DIAGONAL BARS AT CORNERS OF OPENINGS IN SLABS AND CONCRETE WALLS. SEE DETAILS "RECTANGULAR OPENING WALL SLAB" AND "CIRCLE OPENING WALL SLAB". PROVIDE 2" CLEAR COVER BETWEEN THE OPENING AND THE CORNER REINFORCING BARS.
9. WALL FOOTING REINFORCEMENT SHALL BE CONTINUOUS THROUGH COLUMN FOOTING.
10. EXTEND ALL FOOTING REINFORCEMENT TO FAR SIDE OF FOOTING. SEE NOTE BELOW FOR CONCRETE COVERAGE.
11. PROVIDE DOWELS IN WALL FOOTING TO MATCH WALL VERTICALS UNLESS NOTED OTHERWISE ON DRAWINGS. PROVIDE CLASS B SPLICE. USE STANDARD ACI 90 DEGREE HOOK WITH 3" CLEAR TO BOTTOM OF FOOTING UNLESS NOTED OTHERWISE. SEE DETAIL "CORNER BAR & SPLICE LENGTH DETAIL (IN CONCRETE)"

CLASS B SPLICE OR CORNER BAR PER ACI 318

BAR #	3000 PSI CONCRETE		4000 PSI CONCRETE		5000 PSI CONCRETE	
	MIN. SPLICE (INCHES)	MIN. SPLICE (BAR DIAM.)	MIN. SPLICE (INCHES)	MIN. SPLICE (BAR DIAM.)	MIN. SPLICE (INCHES)	MIN. SPLICE (BAR DIAM.)
4	29	57	25	50	24	45
5	36		31		28	
6	43		37		34	
7	63	72	54	62	49	56
8	72		62		56	
9	81		70		63	
10	89		78		69	
11	98		85		76	

12. MINIMUM CONCRETE COVERAGE SHALL BE AS FOLLOWS. IF CONSTRUCTION DOCUMENTS INDICATE A LARGER COVERAGE, IT SHALL BE USED. IF STIRRUPS, TIES, OR SPIRALS ARE USED, COVERAGE SHALL BE TO THE OUTERMOST FACE OF THESE ELEMENTS.
    A. FOOTINGS, CAISSONS, AND OTHER MEMBERS WHERE CONCRETE IS DEPOSITED AGAINST SOIL (EXCEPT SLABS ON GRADE) = 3"
    B. CONCRETE EXPOSED TO WEATHER OR SOIL BUT IS NOT DEPOSITED AGAINST SOIL:
        #6 BAR AND LARGER = 2"
        #5 BAR AND SMALLER = 1 1/2"
    C. CONCRETE NOT EXPOSED TO WEATHER OR SOIL:
        SLABS, WALLS, JOISTS #14 BAR AND LARGER = 1 1/2"
        SLABS, WALLS, JOISTS #11 BAR AND SMALLER = 3/4"
        BEAMS AND COLUMNS = 1 1/2"

STEEL DECK

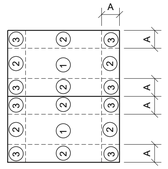
1. MATERIAL, DESIGN, FABRICATION, AND INSTALLATION SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS OF THE STEEL DECK INSTITUTE. (SDI)
2. STEEL DECK SHALL BE CUT TO LENGTHS TO PROVIDE A MINIMUM OF THREE SPAN CONDITION. STEEL DECK SUBMITTAL DRAWINGS SHALL CLEARLY INDICATE BY CLOUDING ONE OR TWO SPAN CONDITIONS.
3. STEEL DECK SHALL BE FASTENED THROUGH THE DECK ONTO THE SUPPORTING MEMBERS AND AT SIDELAPS PER THE DRAWINGS. USE WELDING WASHERS AS REQUIRED TO PREVENT BURN-THROUGH OF STEEL DECK.
4. ALL STEEL ROOF DECK NOT EXPOSED TO WEATHER SHALL BE PRIME PAINTED. ALL STEEL DECK TO RECEIVE CONCRETE TOPPING SHALL BE G60 GALVANIZED IN CONFORMANCE WITH ASTM A653. ALL DECK EXPOSED TO WEATHER SHALL BE G90 GALVANIZED IN CONFORMANCE WITH ASTM A653.
5. STEEL DECK MANUFACTURER SHALL PROVIDE ALL REQUIRED ACCESSORIES.
6. FASTEN ROOF DECK TO STEEL SUPPORTING MEMBERS AT EDGE AND INTERIOR RIBS WITH A SUFFICIENT NUMBER OF 5/8" DIAMETER PUDDLE WELDS FOR A 36/4 PATTERN, AT EDGES PROVIDE 5/8" DIAMETER PUDDLE WELD IN EVERY FLUTE. USE 3 #10 TEK SCREWS PER SIDELAP SPAN IN THE FIELD AND AT PLACES WHERE DECK IS CANTILEVERED.

STEEL JOISTS AND JOIST GIRDERS

1. MATERIAL, DESIGN, FABRICATION, AND INSTALLATION SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS OF THE STEEL DECK INSTITUTE/SDI)
2. TOP AND BOTTOM CHORD BRIDGING SHALL BE SIZED AND SPACED BY THE JOIST MANUFACTURER IN ACCORDANCE WITH SJI SPECIFICATIONS. GENERAL CONTRACTOR COORDINATE MISCELLANEOUS STEEL FOR TERMINATION AND CONNECTION OF BRIDGING AS REQUIRED BY SJI.
3. CONNECT JOISTS AND JOIST GIRDERS TO SUPPORTS PER SJI REQUIREMENTS.
4. DESIGN JOISTS, JOIST GIRDERS, CONNECTIONS AND BRIDGING FOR UPLIFT AS FOLLOWS: A (WIDTH OF ZONE 2 = 3, 'A' = 10.8 FT.)

STEEL JOISTS UPLIFT VALUES

	ZONE	*GROSS UPLIFT PRESSURE (P.S.F.)	NET UPLIFT PRESSURE (P.S.F.)
ROOF	1	22	17
	2	29	24
	3	35	30
OVERHANG	2	33	28
	3	43	38



*DEAD LOAD HAS NOT BEEN ACCOUNTED FOR IN THE VALUES

5. THE DRAWINGS INDICATE LOADS CONSIDERED FOR DESIGN OF JOISTS AND JOIST GIRDERS. ANY CONCENTRATED LOAD IN EXCESS OF 50 POUNDS NOT SHOWN ON THE DRAWINGS SHALL BE COORDINATED WITH THE STRUCTURAL ENGINEER.
6. JOIST TOP CHORD EXTENSIONS SHALL BE DESIGNED BY THE JOIST MANUFACTURER TO SUPPORT A TOTAL DOWNWARD SERVICE LOAD OF 200 POUNDS PER FOOT AND LIMITED TO A MAXIMUM TOTAL LOAD DEFLECTION OF L/240.

STRUCTURAL STEEL

1. DESIGN, DETAILING, FABRICATION AND ERECTION OF STRUCTURAL STEEL SHALL BE IN ACCORDANCE WITH THE CODE-REFERENCED AISC MANUAL OF STEEL CONSTRUCTION, SPECIFICATION FOR STEEL BUILDINGS, AND CODE OF STANDARD PRACTICE.
2. STEEL FABRICATOR SHALL BE CURRENTLY CERTIFIED BY THE AISC QUALITY CERTIFICATION PROGRAM FOR STRUCTURAL STEEL FABRICATIONS AND DESIGNATED AS "AISC CERTIFIED BUILDING FABRICATOR CATEGORY BU." CONTRACTOR SHALL SUBMIT IN WRITING TO THE STRUCTURAL ENGINEER, AT THE TIME OF PROOF OF CERTIFICATION FOR THE STEEL FABRICATOR(S) SUPPLYING STRUCTURAL STEEL.
3. MATERIALS SHALL MEET THE FOLLOWING MINIMUM REQUIREMENTS:
    A. W-SHAPES = ASTM 992
    B. HOLLOW STRUCTURAL SHAPES = ASTM A500, GRADE B
    C. PLATES, BARS, ANGLES, C-SHAPES, MC-SHAPES = ASTM A36
    D. PIPES = ASTM A53, GRADE B
    E. WELDING ELECTRODES = E70XX
4. ALL ANCHOR BOLTS SHALL BE SIZE AND STRENGTH SPECIFIED ON THESE DRAWINGS.
5. ALL BEAM END CONNECTIONS SHALL BE AISC DOUBLE ANGLE BOLTED-WELDED CONNECTIONS WITH 3/4" DIA. A325N BOLTS U.N.O. THE WELD SHALL BE 1/4" WELD FULL LENGTH OF ANGLE PLUS 1" TOP AND BOTTOM. DESIGN SHEAR SHALL BE THE GREATER OF:
    A. THE SHEAR REACTION SHOWN ON DRAWINGS (IF ANY);
    B. 50% OF THE VALUE FROM THE "MAXIMUM TOTAL UNIFORM LOAD IN KIPS" TABLES OF THE AISC 13TH EDITION (BLACK BOOK) OR,
    C. THE MINIMUM NUMBER OF BOLTS IN SINGLE SHEAR AS FOLLOWS:

BEAM SHAPE*	# OF 3/4" DIA. A325 BOLTS	LENGTH OF LL 3 1/2"x3 1/2"x5/16"
W8 , W10	4	5 1/2
W12 , W14	6	8 1/2
W16 , W18	8	11 1/2
W21	10	14 1/2
W24	12	17 1/2
W27	14	20 1/2
W30	16	23 1/2

WHERE CONNECTIONS ARE SKEWED OR THE DOUBLE ANGLE CONNECTIONS ABOVE WILL NOT FIT, THE FOLLOWING CONNECTIONS SHALL BE USED:

END OF BEAM CONNECTIONS*

BEAM SHAPE*	# OF 3/4" DIA. A325 BOLTS	1/2" SHEAR TAB LENGTH**
W8 , W10	2	5 1/2
W12 , W14	3	8 1/2
W16 , W18	4	11 1/2
W21	5	14 1/2
W24	6	17 1/2
W27	7	20 1/2
W30	8	23 1/2

*WHEN THE SHEAR TAB CONNECTION ABOVE DOES NOT FIT IN THE BEAM WEB, USE THE ADJACENT SMALLER CONNECTION AND CLOUD ON SHOP DRAWINGS.

**WELD PLATE TO SUPPORTING MEMBER WITH 5/16" WELD ALL AROUND. SHEAR TAB TO BE 1/2" THICK X 4" WIDE.

6. ALL BOLTED CONNECTION SHALL BE FULLY PRETENSIONED ACCORDING TO THE REQUIREMENTS OF "AISC MANUAL OF STEEL CONSTRUCTION" UTILIZING DIRECT TENSION INDICATORS OR AN APPROVED INSTALLATION METHOD IN WRITING FROM THE ENGINEER OF RECORD.
7. WELDS SHALL BE MADE ONLY BY OPERATORS CERTIFIED BY THE STANDARD QUALIFICATION PROCEDURE OF THE AMERICAN WELDING SOCIETY FOR THE TYPE OF WELD REQUIRED. WELDER CERTIFICATION SHALL BE SUBMITTED FOR REVIEW.
8. WELD LENGTHS NOT NOTED SHALL BE FULL LENGTH. TERMINATE WELDS IN ACCORDANCE WITH AISC MANUAL OF STEEL CONSTRUCTION AND AMERICAN WELDING SOCIETY STRUCTURAL WELDING CODE - STEEL (D1.1).
9. HOLES LARGER THAN 1" DIA. SHALL BE COORDINATED WITH THE STRUCTURAL ENGINEER. HOLES SHALL BE PUNCHED OR DRILLED, EXCEPT AS OTHERWISE PERMITTED THE STRUCTURAL ENGINEER.
10. PROTECT COLUMNS, BASE PLACES, ANCHOR BOLTS, AND ANY STEEL BELOW GRADE WITH AN APPROVED INORGANIC OR EPOXY ANTI-CORROSION COATING, FIELD APPLIED PER MANUFACTURER'S INSTRUCTIONS.
11. ALL EXPOSED STRUCTURAL STEEL INCLUDING LINTELS, AND AS NOTED ON DRAWINGS, SHALL BE GALVANIZED IN CONFORMANCE WITH ASTM A123.FASTENERS AND SMALL PARTS REQUIRING GALVANIZING SHALL BE IN CONFORMANCE WITH ASTM A153.
12. THE CONTRACTORS SHALL DETERMINE, FURNISH AND INSTALL ALL TEMPORARY SUPPORTS SUFFICIENT TO SECURE THE STRUCTURAL STEEL FRAMING AGAINST LOADS PRESENT DURING ERECTION. TEMPORARY SUPPORTS SHALL REMAIN IN PLACE UNTIL ALL CONNECTIONS TO THE LATERAL LOAD RESISTING SYSTEM, INCLUDING HORIZONTAL DIAPHRAGMS, ARE COMPLETE.
13. THE GENERAL CONTRACTOR SHALL VERIFY THAT THE CORRECT BEAM AND GIRDER CAMBER IS PRESENT AFTER ERECTION AND BEFORE FLOOR SLAB IS POURED.
14. SPLICE CONTINUOUS STEEL ANGLES AND PLATES WITH PARTIAL-JOINT-PENETRATION SQUARE GROOVE WELDS (JOINT DESIGNATION B-P1A) U.N.O.
15. STRUCTURAL STEEL FABRICATOR AND DETAILER SHALL SEE THE ARCHITECTURAL DRAWINGS FOR ANY ADDITIONAL STEEL NOT SHOWN OR CALLED OUT IN THESE DRAWINGS. IF SIZE IS NOT SHOWN IN ARCHITECTURAL DRAWINGS A REQUEST OR INFORMATION SHALL BE SENT TO THE STRUCTURAL ENGINEER THROUGH THE PROPER CHANNELS.
16. GENERAL CONTRACTOR SHALL COORDINATE CONNECTIONS OF JOIST AND JOIST GIRDERS TO STRUCTURAL STEEL.

CONCRETE BLOCK LINTEL BEAM SCHEDULE

SPAN	LINTEL	
UP TO 3'-0"	1 #4 T & 1 #5 B	1 COURSE DEEP
3'-1" TO 5'-0"	1 #5 T & 1 #6 B	
5'-1" TO 7'-0"	1 #6 T & 1 #7 B	2 COURSES DEEP
7'-1" TO 9'-0"	1 #8 T & 1 #9 B	

NOTES:
1) LINTELS SCHEDULED ABOVE SHALL BE USED UNLESS SHOWN OR NOTED OTHERWISE.
2) LINTELS SHALL HAVE 1" BEARING, EACH END, FOR EACH 1'-0" OF SPAN, BUT NOT LESS THAN 8" BEARING EACH END.
3) CONCRETE BLOCK LINTELS SHALL BE MADE WITH FILLED "U" BLOCKS. FILL SHALL BE MADE WITH COURSE GROUT, CONFORMING TO ASTM C-476, 3/8" MAX. STONE SIZE. SHORE POURED LINTELS (7) DAYS MINIMUM.
4) REINFORCING SHOWN ABOVE SHALL BE DOUBLED FOR 12" NOM. CONCRETE BLOCK LINTELS.

GEORGIA REGISTERED No. PE011263 PROFESSIONAL ENGINEER GARY VAN KORNEGAY
5-16-2025

DUNWODY/BEELAND, Architects, Inc.
300 Mulberry Street, Suite 604
Macon, GA 31201
T 478.742.5321
F 478.743.3863
www.dunwodybeeland.com

OFFICE RENOVATION FOR VILTIES HOLDINGS LLC

4951 FORSYTH ROAD, MACON, GA 31210

Revisions:	

Sheet Title:

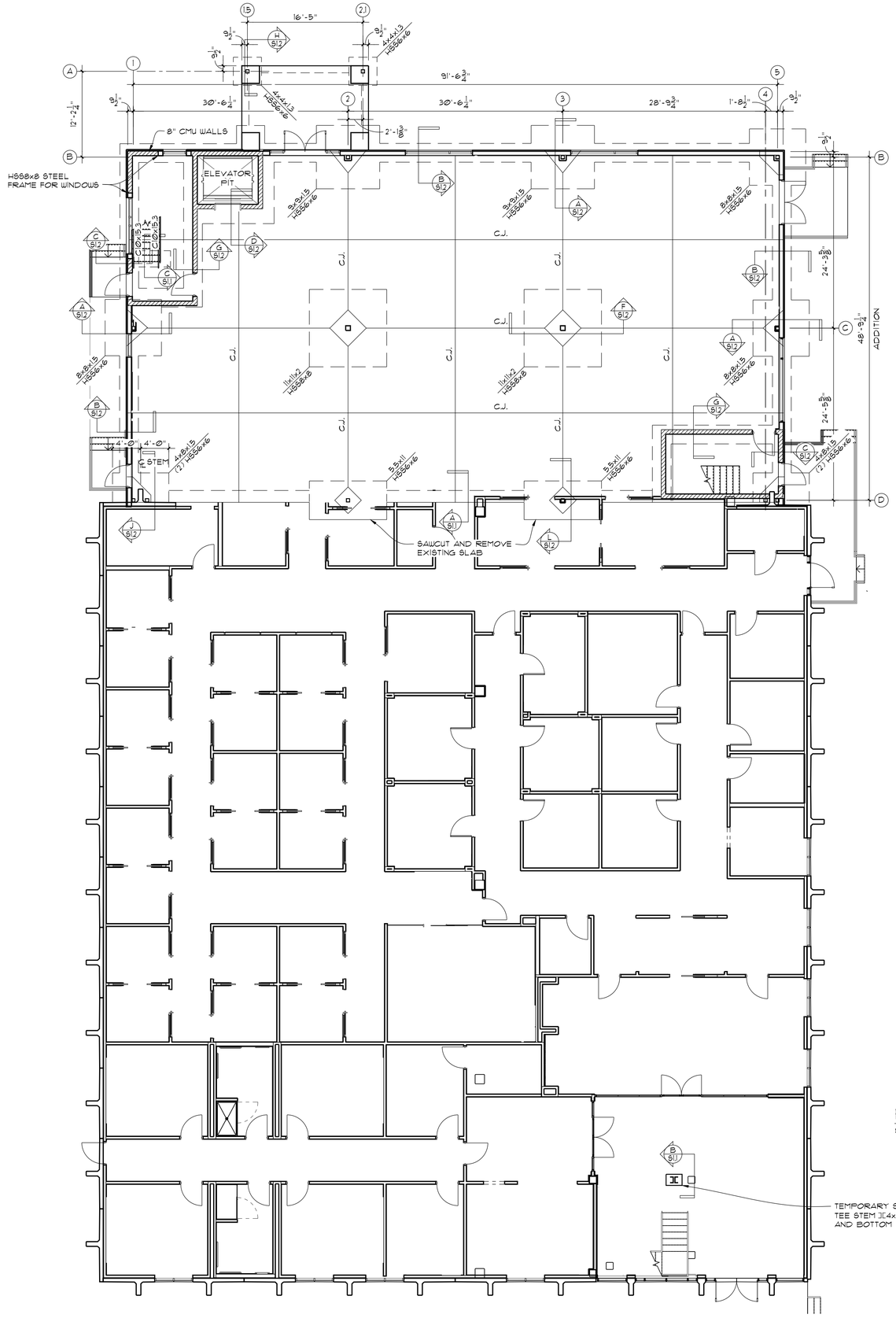
GENERAL NOTES

Kornegay Engineering Inc.
Structural Consultant
363 Pierce Avenue
Suite 202
Macon, GA 31204
(478)745-6161 ph

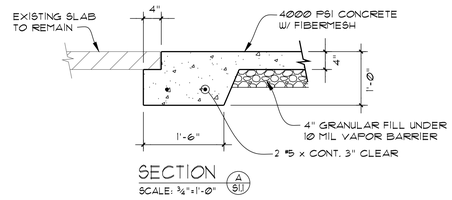
Project No:24-311

Project #: 2229 **Date:** 3/7/2025

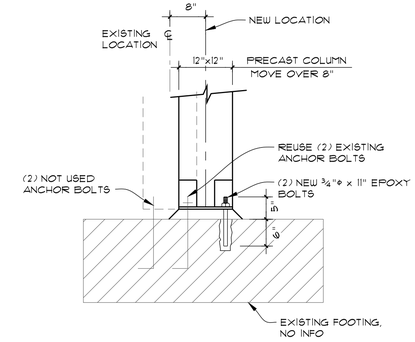
S0.1



FOUNDATION PLAN
SCALE: 1/8"=1'-0"

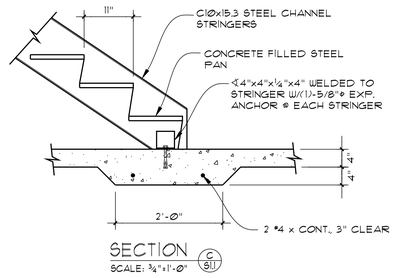


SECTION
SCALE: ¾"=1'-0"



SECTION
SCALE: ¾"=1'-0"

NOTE: AFTER FOOTING HAS BEEN EXPOSED, STRUCTURAL ENGINEER IS TO BE NOTIFIED.



SECTION
SCALE: ¾"=1'-0"

Kornegay Engineering Inc.
Structural Consultant
363 Pierce Avenue
Suite 202
Macon, GA 31204
(478)745-6161 ph
Project No:24-311

GEORGIA REGISTERED No. PE011263 PROFESSIONAL ENGINEER HARRY DAN KORNEGAY
5-16-2025

DUNWODY/BEELAND, Architects, Inc.
300 Mulberry Street, Suite 604
Macon, GA 31201
T 478.742.5321
F 478.743.0863
www.dunwodybeeland.com

OFFICE RENOVATION FOR VILTIES HOLDINGS LLC

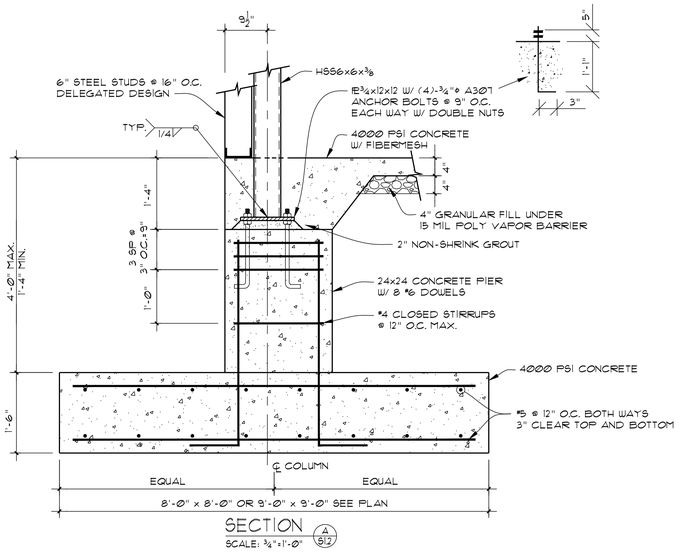
4951 FORSYTH ROAD, MACON, GA 31210

Revisions:

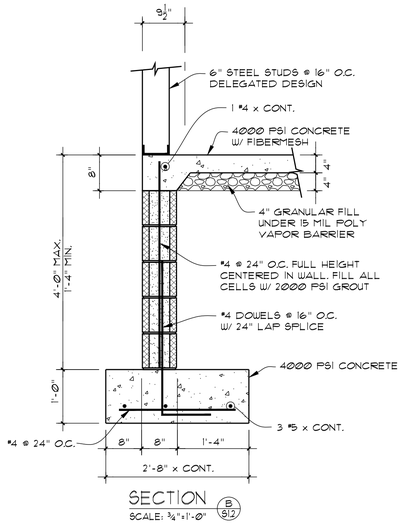
Sheet Title:
FOUNDATION PLAN

Project #: 2229
Date: 3/7/2025

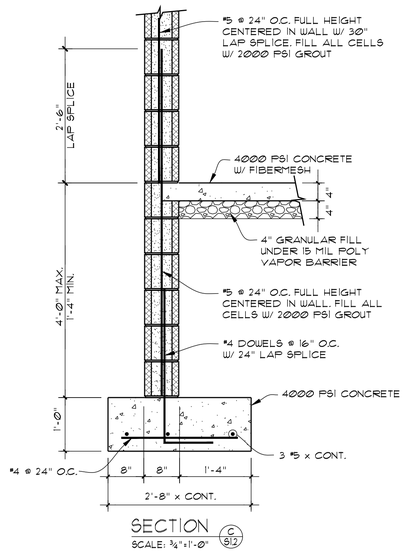
S1.1



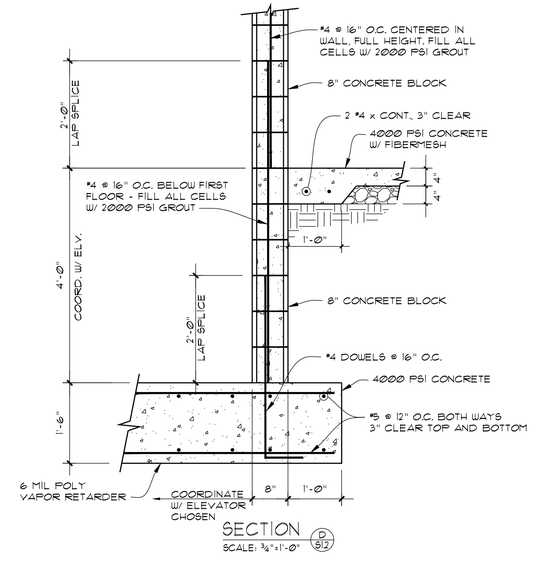
SECTION (A S1.2)
SCALE: ¾"=1'-Ø"



SECTION (B S1.2)
SCALE: ¾"=1'-Ø"



SECTION (C S1.2)
SCALE: ¾"=1'-Ø"

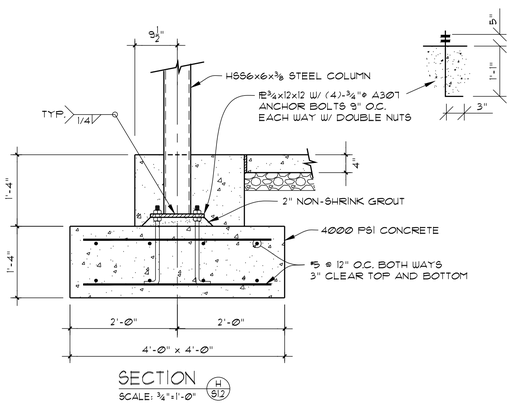


SECTION (D S1.2)
SCALE: ¾"=1'-Ø"

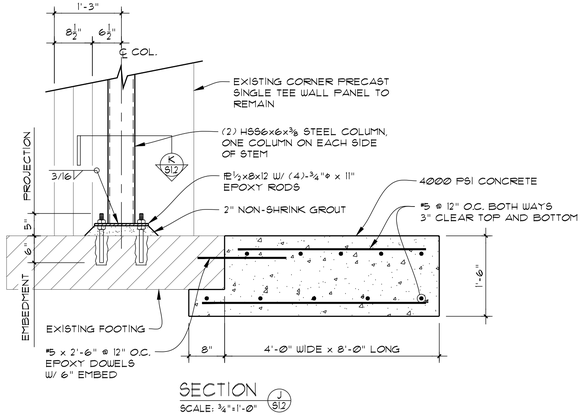
SUMP DETAIL (E S1.2)
SCALE: ¾"=1'-Ø"

SECTION (F S1.2)
SCALE: ¾"=1'-Ø"

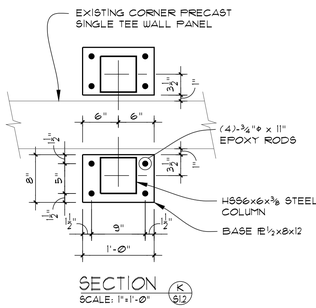
SECTION (G S1.2)
SCALE: ¾"=1'-Ø"



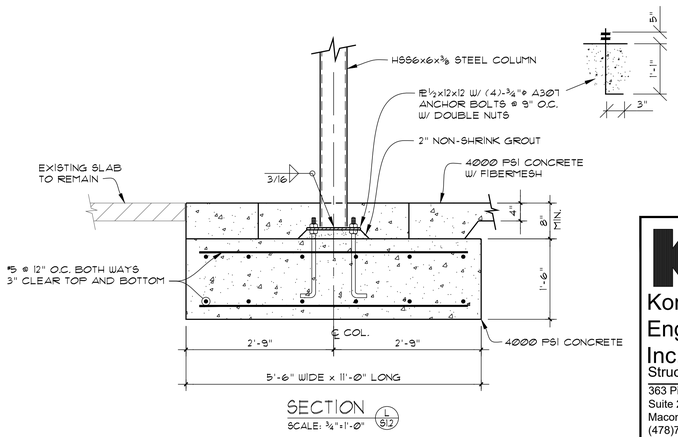
SECTION (H S1.2)
SCALE: ¾"=1'-Ø"



SECTION (J S1.2)
SCALE: ¾"=1'-Ø"



SECTION (K S1.2)
SCALE: 1"=1'-Ø"



SECTION (L S1.2)
SCALE: ¾"=1'-Ø"

GEORGIA REGISTERED PROFESSIONAL No. PE011263 ENGINEER BARRY DAN TOPHAM
5-16-2025

DUNWODY/BEELAND, Architects, Inc.
300 Mulberry Street, Suite 604
Macon, GA 31201
T 478.742.5321
F 478.743.0863
www.dunwodybeeland.com

OFFICE RENOVATION FOR VILTIES HOLDINGS LLC

4951 FORSYTH ROAD, MACON, GA 31210

Revisions:

Sheet Title:

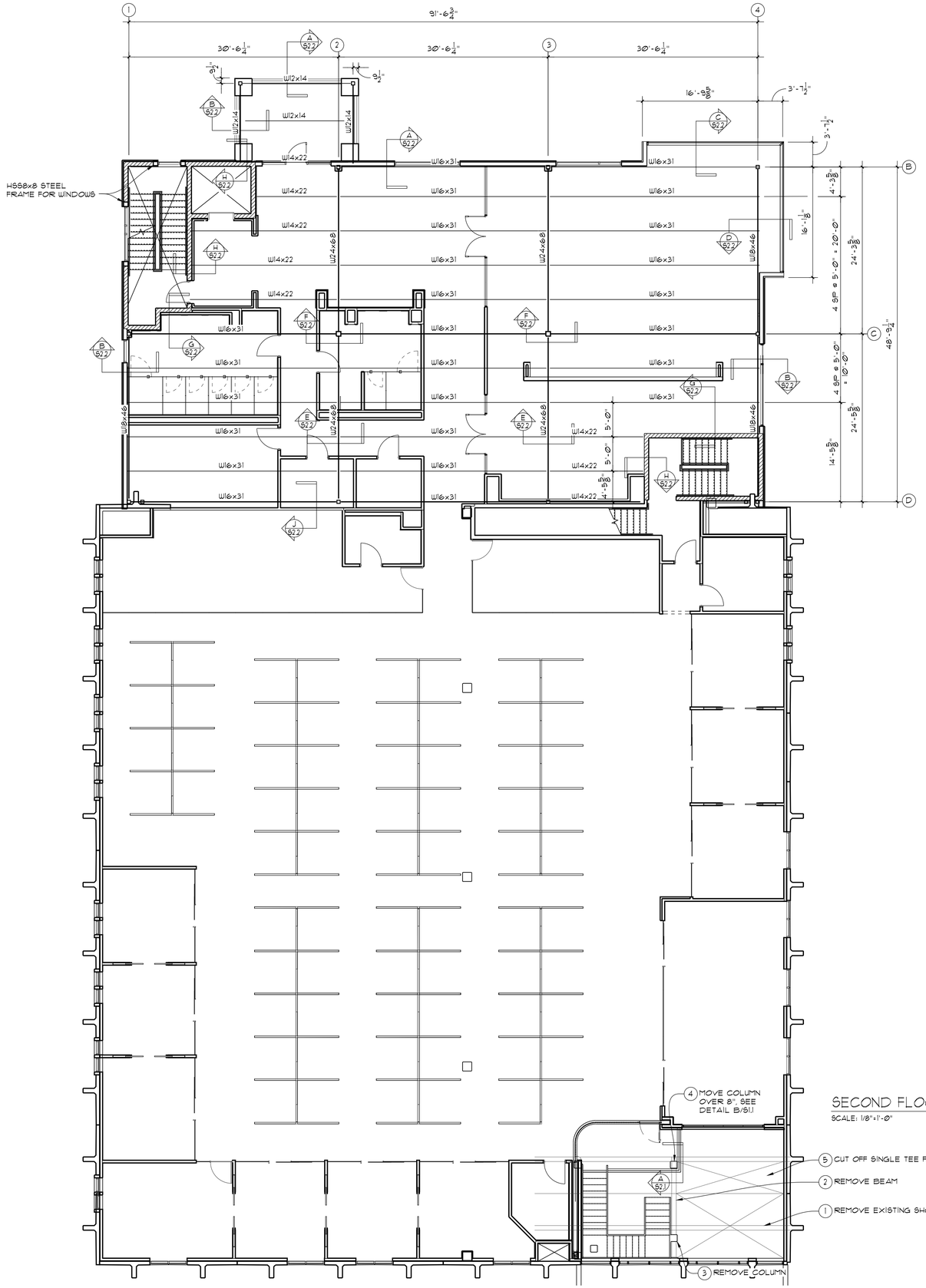
FOUNDATION SETIONS

Project #: 2229 **Date:** 3/7/2025

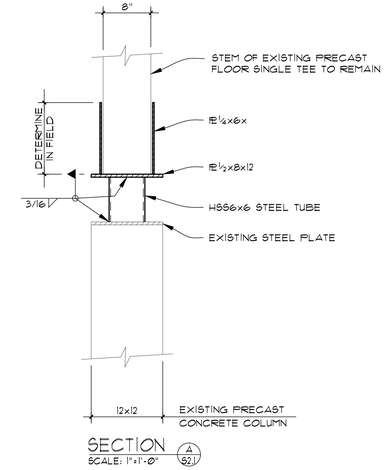
S1.2

Kornegay Engineering Inc.
Structural Consultant
363 Pierce Avenue
Suite 202
Macon, GA 31204
(478)745-6161 ph

Project No:24-311



SECOND FLOOR FRAMING PLAN
SCALE: 1/8"=1'-0"



SECTION
SCALE: 1"=1'-0"

GEORGIA REGISTERED No. PE011263 PROFESSIONAL ENGINEER GARRY DAN KORNEGAY
5-16-2025

DUNWODY/BEELAND, Architects, Inc.
300 Mulberry Street, Suite 604
Macon, GA 31201
T 478.742.5321
F 478.743.0863
www.dunwodybeeland.com

OFFICE RENOVATION FOR VILTIES HOLDINGS LLC

4951 FORSYTH ROAD, MACON, GA 31210

Revisions:

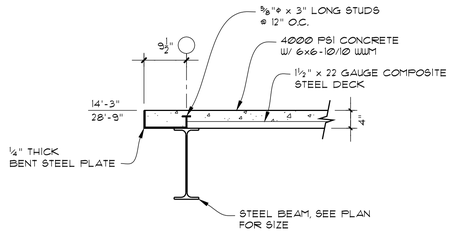
Sheet Title:

SECOND FLOOR FRAMING PLAN AND SECTIONS

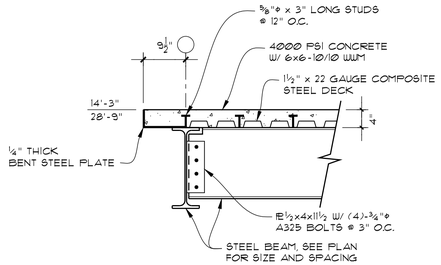
Project #: 2229 **Date:** 3/7/2025

Kornegay Engineering Inc.
Structural Consultant
363 Pierce Avenue
Suite 202
Macon, GA 31204
(478)745-6161 ph
Project No:24-311

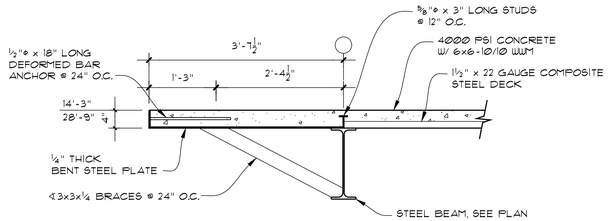
S2.1



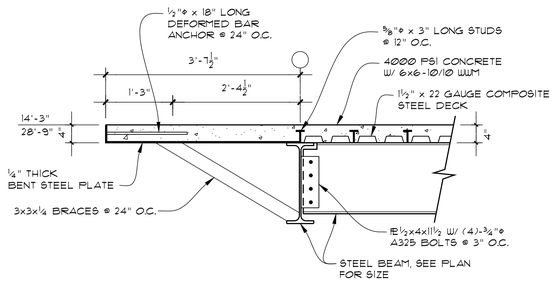
SECTION
SCALE: ¾"=1'-0" (A S2.2)



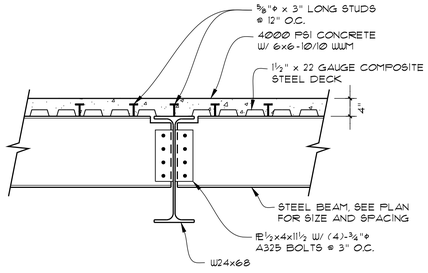
SECTION
SCALE: ¾"=1'-0" (B S2.2)



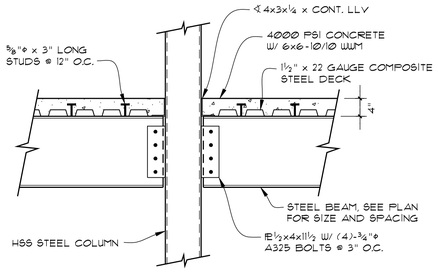
SECTION
SCALE: ¾"=1'-0" (C S2.2)



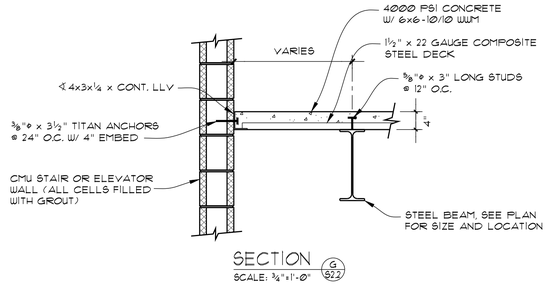
SECTION
SCALE: ¾"=1'-0" (D S2.2)



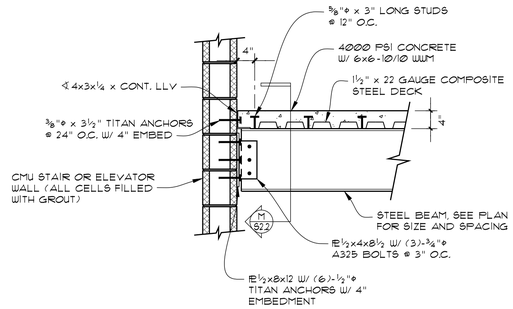
SECTION
SCALE: ¾"=1'-0" (E S2.2)



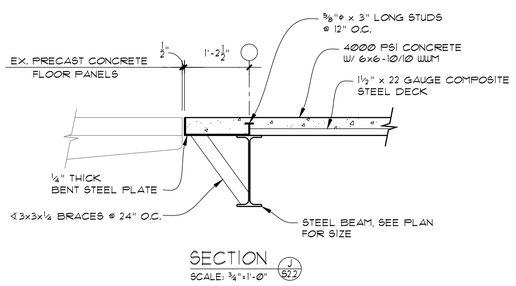
SECTION
SCALE: ¾"=1'-0" (F S2.2)



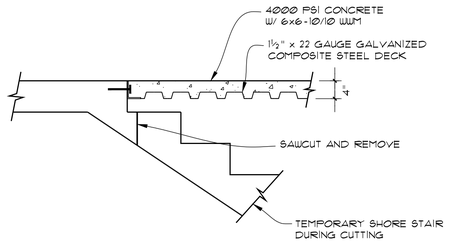
SECTION
SCALE: ¾"=1'-0" (G S2.2)



SECTION
SCALE: ¾"=1'-0" (H S2.2)



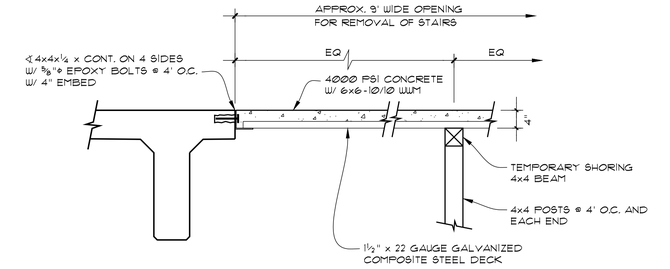
SECTION
SCALE: ¾"=1'-0" (J S2.2)



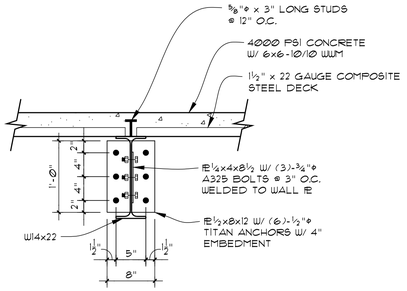
SEQUENCE OF EVENTS FOR REMOVAL OF EXISTING CONCRETE STAIRS:

1 - ADEQUATELY SHORE STAIRS
2 - SAWCUT AND REMOVE STAIRS
3 - INSTALL ∢4x4x¼ x CONT. ON 4 SIDES AND INSTALL COMPOSITE STEEL DECK
4 - PROVIDE TEMPORARY WOOD SHORING @ MIDSPAN OF STEEL DECKING BEFORE PLACING CONCRETE

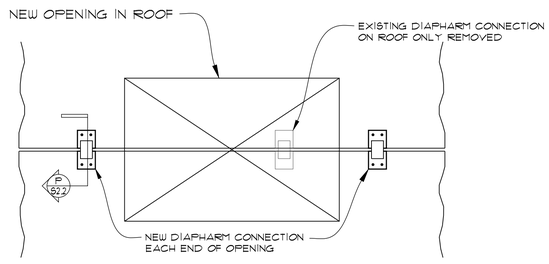
SECTION
SCALE: ¾"=1'-0" (K S2.2)



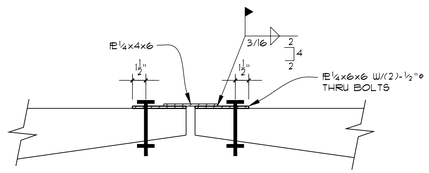
SECTION
SCALE: ¾"=1'-0" (L S2.2)



SECTION
SCALE: 1"=1'-0" (M S2.2)



ROOF OPENING DETAIL
SCALE: 1/2"=1'-0" (N S2.2)



SECTION
SCALE: 1½"=1'-0" (P S2.2)

GEORGIA REGISTERED No. PE011263 PROFESSIONAL ENGINEER JERRY VAN KOPPEN
5-16-2025

DUNWODY/BEELAND, Architects, Inc.
300 Mulberry Street, Suite 604
Macon, GA 31201
T 478.742.5321
F 478.743.0863
www.dunwodybeeland.com

OFFICE RENOVATION FOR VILTIES HOLDINGS LLC

4951 FORSYTH ROAD, MACON, GA 31210

Revisions:

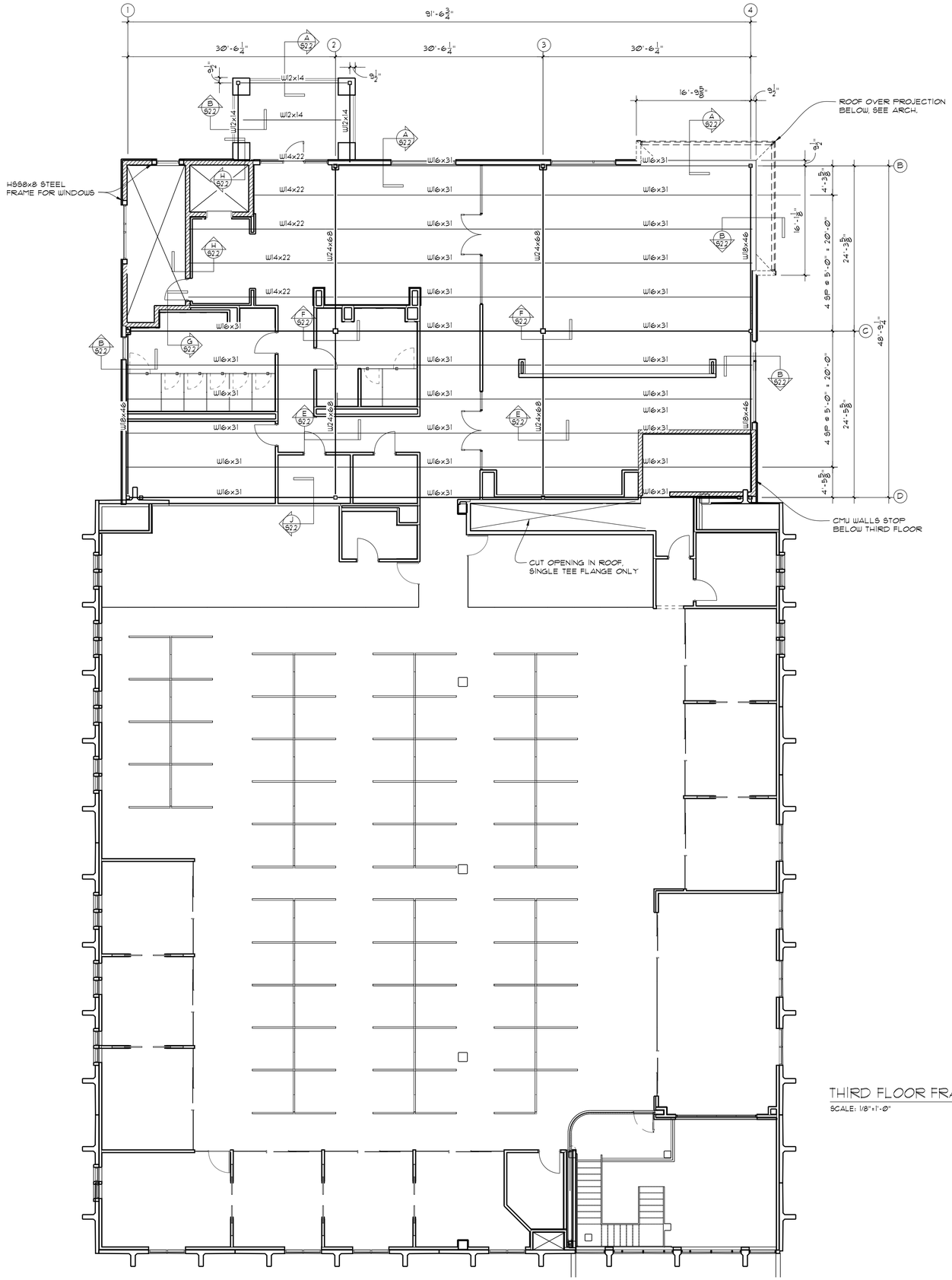
Sheet Title:

SECOND FLOOR FRAMING SECTIONS

Kornegay Engineering Inc.
Structural Consultant
363 Pierce Avenue
Suite 202
Macon, GA 31204
(478)745-6161 ph
Project No:24-311

Project #: 2229 **Date:** 3/7/2025

S2.2



THIRD FLOOR FRAMING PLAN (1/S3.1)

SCALE: 1/8"=1'-0"

GEORGIA REGISTERED No. PE011263 PROFESSIONAL ENGINEER GARY DAN KORNEGAY

5-16-2025

DUNWODY/BEELAND, Architects, Inc.

300 Mulberry Street, Suite 604
Macon, GA 31201
T 478.742.5321
F 478.743.0863
www.dunwodybeeland.com

OFFICE RENOVATION FOR VILTIES HOLDINGS LLC

4951 FORSYTH ROAD, MACON, GA 31210

Revisions:

Sheet Title:

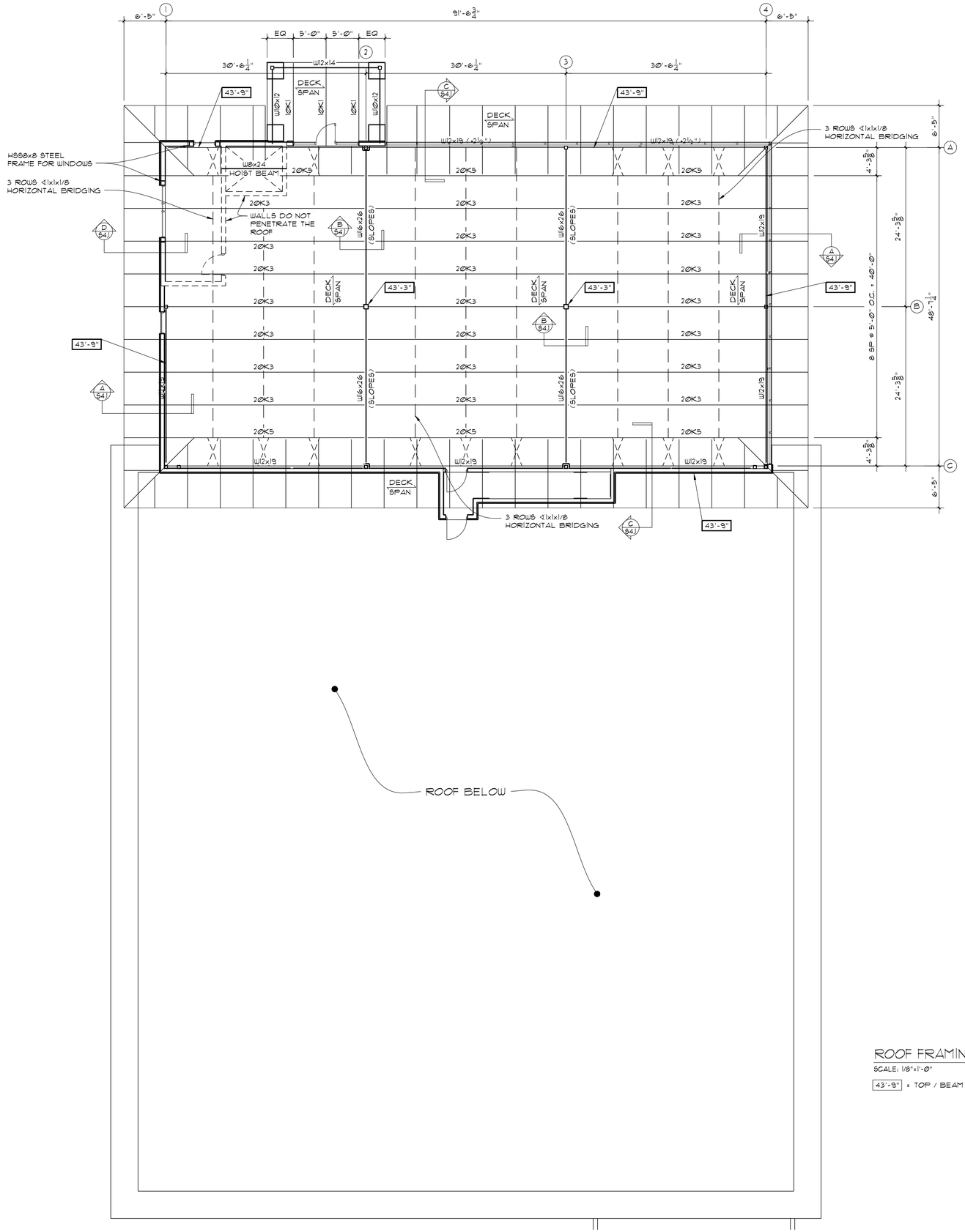
THIRD FLOOR FRAMING PLAN

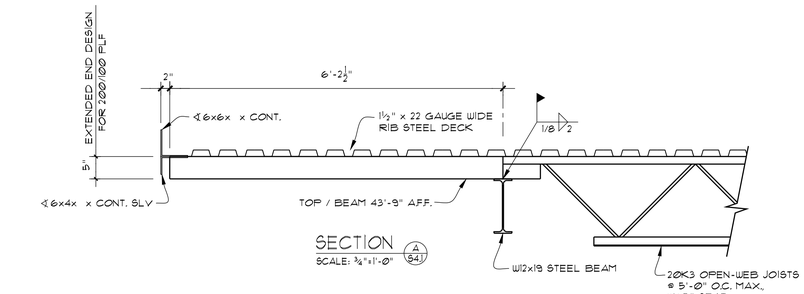
Kornegay Engineering Inc.
Structural Consultant
363 Pierce Avenue
Suite 202
Macon, GA 31204
(478)745-6161 ph

Project No:24-311

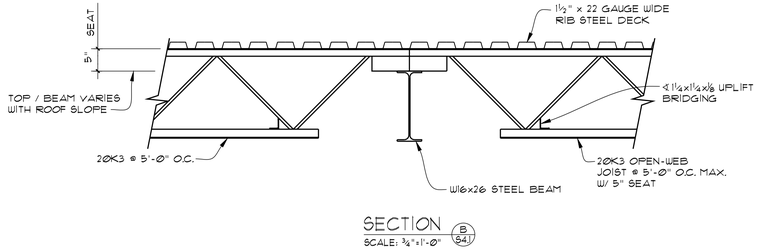
Project #: 2229 **Date:** 3/7/2025

S3.1

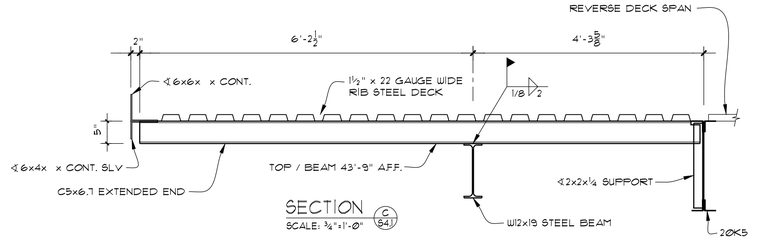




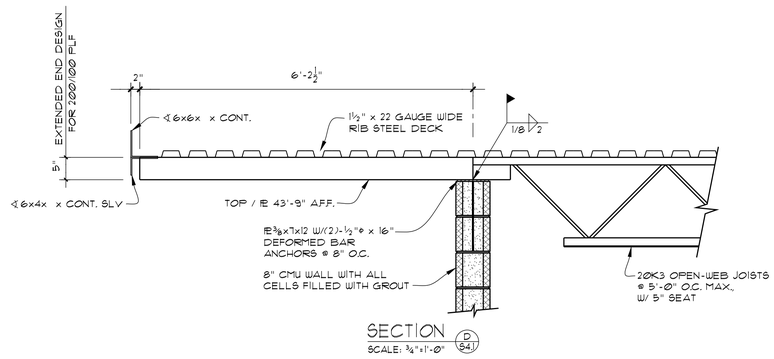
SECTION
SCALE: ¾"=1'-Ø" A S4.1



SECTION
SCALE: ¾"=1'-Ø" B S4.1



SECTION
SCALE: ¾"=1'-Ø" C S4.1



SECTION
SCALE: ¾"=1'-Ø" D S4.1

ROOF FRAMING PLAN
SCALE: 1/8"=1'-Ø" 1 S4.1

43'-9" = TOP / BEAM A.F.F.

Kornegay Engineering Inc.
Structural Consultant
363 Pierce Avenue
Suite 202
Macon, GA 31204
(478)745-6161 ph
Project No:24-311

GEORGIA REGISTERED No. PE011263 PROFESSIONAL ENGINEER HARRY DAN KORNEGAY
5-16-2025

DUNWODY/BEELAND, Architects, Inc.
300 Mulberry Street, Suite 604
Macon, GA 31201
T 478.742.5321
F 478.743.0883
www.dunwodybeeland.com

OFFICE RENOVATION FOR VILTIES HOLDINGS LLC

4951 FORSYTH ROAD, MACON, GA 31210

Revisions:

Sheet Title:

ROOF FRAMING PLAN

Project #: 2229 **Date:** 3/7/2025

S4.1